

Diamond Wood & Shaw Limited

The Old School · Blaby Road · Enderby · Leicester · LE19 4AR
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Drainage Calculations

Client: M2 Properties Ltd

Architect: Staniforth Architects

Project: 68 Wood Street, Earl Shilton

Project No: 25-22559

Document No: 0-010

Issue: Rev P1: December 2025

Status: Preliminary

Date: 09/12/2025

Engineer: LM

Issue Status:
A=Approval
B=Building Regulations
C=Construction
D=Draft
P=Preliminary
R=Review
T=Tender
Z=Final Construction

Total Site Area = 0.132 ha (A)

Total Impermeable Area = 0.132 ha (Ai)

Modified rational method for site peak discharge rate (Qp)

$$Q_p = 2.78 \times C_r \times C_v \times I \times A_i$$

Where,

C_r = Dimensionless routing coefficient (taken as 1.3)

C_v = Volumetric runoff coefficient = P_r / P_{imp}

P_r = Percentage runoff = $(0.829 \times P_{imp}) + (25 \times SOIL) + (0.078 \times UCWI) - 20.7$
(equation 6.4 from Urban Drainage 2nd Ed: Butler et al)

P_{imp} = % Impervious area = $(A_i / A) \times 100 = \frac{0.132}{0.132} \times 100 = 100 \%$

$SOIL$ = Soil Index = 0.45 (from MicroDrainage)

$UCWI$ = Urban Catchment Wetness Index = 130 (fig 6.2 Urban Drainage 2nd)

Therefore, $P_r = (0.829 \times 100) + (25 \times 0.45) + (0.078 \times 130) - 20.7$
 $= 82.9 + 11.25 + 10.14 - 20.7$
 $= 83.59$

Check, $P_r > 0.4 \times P_{imp}$ So, $83.59 > 40$ so, use P_r from equation above

So, if needed, $P_r = 0.4 \times 100 = N/A$

Therefore, $C_v = \frac{83.59}{100} = 0.836$

I = Rainfall intensity (taken from MicroDrainage)

1 in 1 year = 57.09
 1 in 30 year = 133.76
 1 in 100 year = 168.87

1 in 1 $Q_p = 2.78 \times 1.3 \times 0.836 \times 57.09 \times 0.132 = 22.77$ l/s
 1 in 30 $Q_p = 2.78 \times 1.3 \times 0.836 \times 133.8 \times 0.132 = 53.34$ l/s
 1 in 100 $Q_p = 2.78 \times 1.3 \times 0.836 \times 168.9 \times 0.132 = 67.34$ l/s

Proposed rates

Return	Existing	Proposed (50% betterment)
1 in 1	22.77 l/s	11.38 l/s
1 in 30	53.34 l/s	26.67 l/s
1 in 100	67.34 l/s	33.67 l/s

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Manhole Schedules for Storm

MH Name	MH CL (m)	MH Depth (m)	MH Connection	MH Diam., L*W (mm)	PN	Pipe Out Invert Level (m)	Diameter (mm)	PN	Pipes In Invert Level (m)	Diameter (mm)	Backdrop (mm)
S1	114.020	0.950	Junction		1.000	113.070	100				
S2	114.020	1.150	Open Manhole	225	1.001	112.870	100	1.000	112.870	100	
S3	114.050	1.350	Open Manhole	450	1.002	112.700	100	1.001	112.700	100	
S4	114.170	1.000	Junction		2.000	113.170	150				
S5	114.000	1.500	Open Manhole	450	1.003	112.500	150	1.002	112.550	100	
								2.000	112.500	150	
S6	111.050	1.550	Open Manhole	450	1.004	109.500	150	1.003	109.500	150	
S7	111.050	1.850	Open Manhole	450	1.005	109.200	150	1.004	109.200	150	
S8	110.600	1.900	Open Manhole	1200	3.000	108.700	150				
S9	110.550	2.200	Open Manhole	1200	1.006	108.350	150	1.005	108.350	150	
								3.000	108.350	150	
F7	110.550	2.380	Open Manhole	0		OUTFALL		1.006	108.170	150	

No coordinates have been specified, layout information cannot be produced.

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PIPELINE SCHEDULES for Storm

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
1.000	o	100	S1	114.020	113.070	0.850	Junction	
1.001	o	100	S2	114.020	112.870	1.050	Open Manhole	225
1.002	o	100	S3	114.050	112.700	1.250	Open Manhole	450
2.000	o	150	S4	114.170	113.170	0.850	Junction	
1.003	o	150	S5	114.000	112.500	1.350	Open Manhole	450
1.004	o	150	S6	111.050	109.500	1.400	Open Manhole	450
1.005	o	150	S7	111.050	109.200	1.700	Open Manhole	450
3.000	o	150	S8	110.600	108.700	1.750	Open Manhole	1200
1.006	o	150	S9	110.550	108.350	2.050	Open Manhole	1200

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
1.000	17.700	88.5	S2	114.020	112.870	1.050	Open Manhole	225
1.001	15.000	88.2	S3	114.050	112.700	1.250	Open Manhole	450
1.002	3.200	21.3	S5	114.000	112.550	1.350	Open Manhole	450
2.000	13.500	20.1	S5	114.000	112.500	1.350	Open Manhole	450
1.003	34.200	11.4	S6	111.050	109.500	1.400	Open Manhole	450
1.004	9.200	30.7	S7	111.050	109.200	1.700	Open Manhole	450
1.005	15.500	18.2	S9	110.550	108.350	2.050	Open Manhole	1200
3.000	13.500	38.6	S9	110.550	108.350	2.050	Open Manhole	1200
1.006	2.500	13.9	F7	110.550	108.170	2.230	Open Manhole	0

Free Flowing Outfall Details for Storm

Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)
1.006	F7	110.550	108.170	0.000	0	0

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Online Controls for Storm

Hydro-Brake® Optimum Manhole: S9, DS/PN: 1.006, Volume (m³): 3.0

Unit Reference	MD-SHE-0142-1140-1800-1140
Design Head (m)	1.800
Design Flow (l/s)	11.4
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	142
Invert Level (m)	108.350
Minimum Outlet Pipe Diameter (mm)	225
Suggested Manhole Diameter (mm)	1500

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.800	11.3
Flush-Flo™	0.530	11.3
Kick-Flo®	1.101	9.0
Mean Flow over Head Range	-	9.9

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	5.1	1.200	9.4	3.000	14.5	7.000	21.7
0.200	9.7	1.400	10.1	3.500	15.6	7.500	22.4
0.300	10.7	1.600	10.7	4.000	16.6	8.000	23.1
0.400	11.2	1.800	11.3	4.500	17.5	8.500	23.8
0.500	11.3	2.000	11.9	5.000	18.4	9.000	24.5
0.600	11.3	2.200	12.5	5.500	19.3	9.500	25.1
0.800	10.9	2.400	13.0	6.000	20.1		
1.000	10.0	2.600	13.5	6.500	20.9		

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Storage Structures for Storm

Cellular Storage Manhole: S6, DS/PN: 1.004

Invert Level (m) 109.500 Safety Factor 2.0
Infiltration Coefficient Base (m/hr) 0.00000 Porosity 0.95
Infiltration Coefficient Side (m/hr) 0.00000

Depth (m)	Area (m ²)	Inf. Area (m ²)	Depth (m)	Area (m ²)	Inf. Area (m ²)
0.000	28.0	28.0	0.801	0.0	46.4
0.800	28.0	46.4			

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1 year Return Period Summary of Critical Results by Maximum Outflow (Rank
1) for Storm

PN	US/MH Name	Surcharged		Flooded		Half Drain		Pipe	Status	Level Exceeded
		Depth (m)	Volume (m³)	Flow / Cap.	Overflow (l/s)	Time (mins)	Flow (l/s)	Flow		
1.002	S3	-0.073	0.000	0.16			1.7		OK	
2.000	S4	-0.130	0.000	0.04			1.7		OK*	
1.003	S5	-0.105	0.000	0.19			9.9		OK	
1.004	S6	-0.088	0.000	0.36		9	10.3		OK	
1.005	S7	-0.096	0.000	0.28			11.0		OK	
3.000	S8	-0.126	0.000	0.06			1.6		OK	
1.006	S9	0.159	0.000	0.41			10.8		SURCHARGED	

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30 year Return Period Summary of Critical Results by Maximum Outflow (Rank 1) for Storm

PN	US/MH Name	Surcharged		Flooded		Half Drain		Pipe	Status	Level Exceeded
		Depth (m)	Volume (m³)	Flow / Cap.	Overflow (l/s)	Time (mins)	Flow (l/s)			
1.002	S3	-0.056	0.000	0.39			4.2		OK	
2.000	S4	-0.117	0.000	0.11			4.3		OK*	
1.003	S5	-0.069	0.000	0.56			28.6		OK	
1.004	S6	0.111	0.000	0.78		11	22.1	SURCHARGED		
1.005	S7	0.336	0.000	0.58			22.3	SURCHARGED		
3.000	S8	0.842	0.000	0.17			4.5	SURCHARGED		
1.006	S9	0.894	0.000	0.43			11.3	SURCHARGED		

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100 year Return Period Summary of Critical Results by Maximum Outflow (Rank 1) for Storm

Simulation Criteria

Areal Reduction Factor 1.000

Hot Start (mins) 0

Hot Start Level (mm) 0

Manhole Headloss Coeff (Global) 0.500

Foul Sewage per hectare (l/s) 0.000

Additional Flow - % of Total Flow 0.000

MADD Factor * 10m³/ha Storage 2.000

Inlet Coeffiecient 0.800

Flow per Person per Day (l/per/day) 0.000

Number of Input Hydrographs 0

Number of Online Controls 1

Number of Offline Controls 0

Number of Storage Structures 1

Number of Time/Area Diagrams 0

Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR

Region England and Wales Cv (Summer) 0.750

M5-60 (mm) 19.400

Ratio R 0.405

Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0

Analysis Timestep 2.5 Second Increment (Extended)

DTS Status ON

DVD Status OFF

Inertia Status OFF

Profile(s) Summer and Winter

Duration(s) (mins) 15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440, 2160, 2880, 4320, 5760, 7200, 8640, 10080

Return Period(s) (years) 1, 30, 100

Climate Change (%) 0, 0, 40

	US/MH		Return	Climate	First (X)	First (Y)	First (Z)	Overflow	Water
PN	Name	Storm	Period	Change	Surcharge	Flood	Overflow	Act.	Level
1.000	S1 15 Winter		100	+40%					113.170
1.001	S2 15 Winter		100	+40%	100/15 Summer				113.009
1.002	S3 15 Winter		100	+40%					112.758
2.000	S4 15 Winter		100	+40%					113.215
1.003	S5 15 Summer		100	+40%	100/15 Summer				112.673
1.004	S6 15 Summer		100	+40%	30/15 Summer				110.081
1.005	S7 15 Summer		100	+40%	30/15 Summer				110.050
3.000	S8 30 Summer		100	+40%	30/15 Summer				110.059
1.006	S9 30 Winter		100	+40%	1/15 Summer				110.171

Surcharged Flooded				Half Drain		Pipe		Level
PN	US/MH	Depth	Volume	Flow /	Overflow	Time	Flow	Level
	Name	(m)	(m³)	Cap.	(l/s)	(mins)	(l/s)	Exceeded
1.000	S1	0.000	0.000	1.08			6.9 SURCHARGED*	
1.001	S2	0.039	0.000	1.11			6.8 SURCHARGED	

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