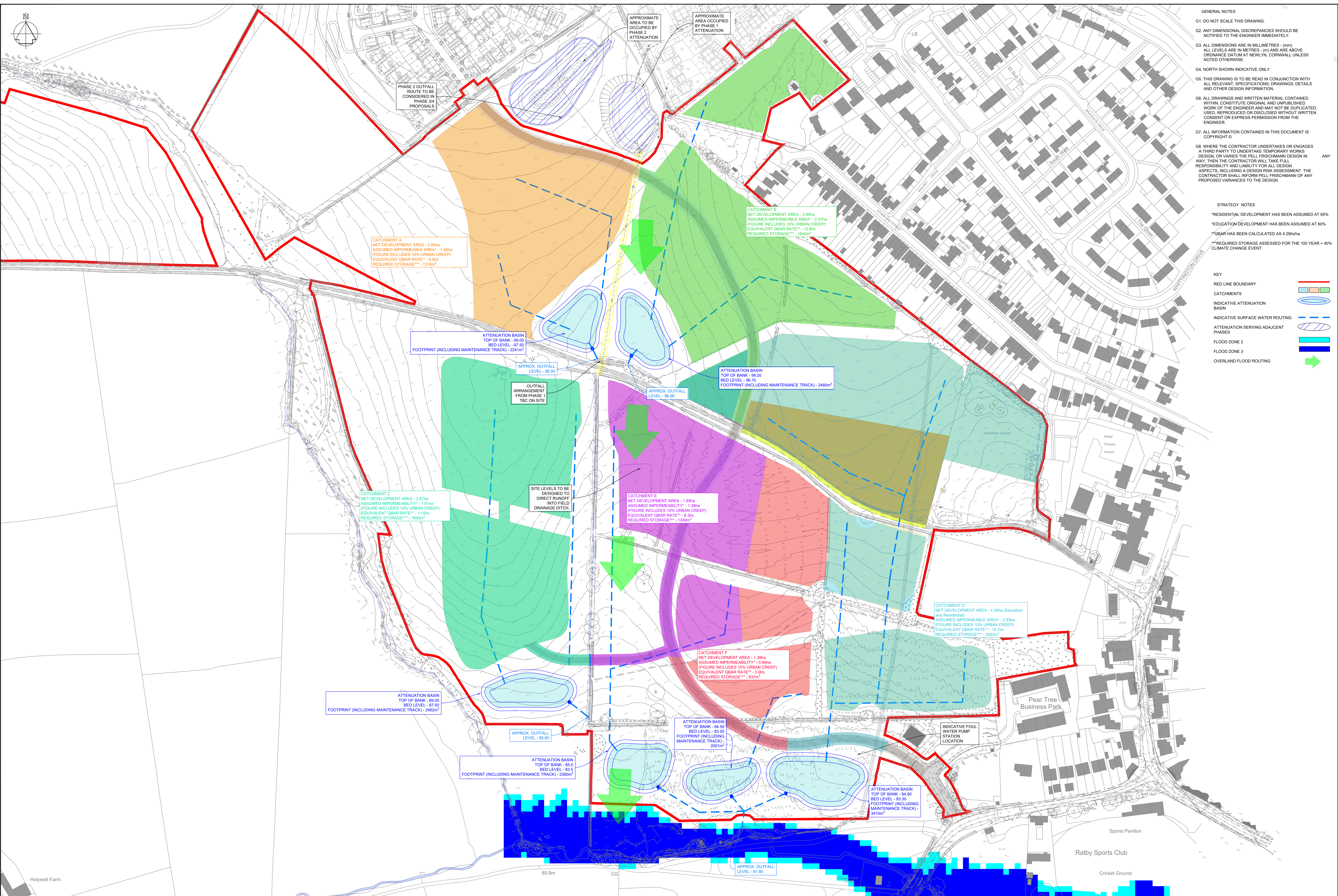




- GENERAL NOTES
- G1. DO NOT SCALE THIS DRAWING.
- G2. ANY DIMENSIONAL DISCREPANCIES SHOULD BE NOTIFIED TO THE ENGINEER IMMEDIATELY.
- G3. ALL DIMENSIONS ARE IN MILLIMETRES - (mm)
ALL LEVELS ARE IN METRES - (m) AND ARE ABOVE ORDNANCE DATUM AT NEWLYN, CORNWALL UNLESS NOTED OTHERWISE.
- G4. NORTH SHOWN INDICATIVE ONLY.
- G5. THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL RELEVANT SPECIFICATIONS, DRAWINGS, DETAILS AND OTHER DESIGN INFORMATION.
- G6. ALL DRAWINGS AND WRITTEN MATERIAL CONTAINED WITHIN, CONSTITUTE ORIGINAL AND UNPUBLISHED WORK OF THE ENGINEER AND MAY NOT BE DUPLICATED, USED, REPRODUCED OR DISCLOSED WITHOUT WRITTEN CONSENT OR EXPRESS PERMISSION FROM THE ENGINEER.
- G7. ALL INFORMATION CONTAINED IN THIS DOCUMENT IS COPYRIGHT ©
- G8. WHERE THE CONTRACTOR UNDERTAKES OR ENGAGES A THIRD PARTY TO UNDERTAKE TEMPORARY WORKS DESIGN, OR VARIES THE PELL FRISCHMANN DESIGN IN ANY WAY, THEN THE CONTRACTOR WILL TAKE FULL RESPONSIBILITY AND LIABILITY FOR ALL DESIGN ASPECTS, INCLUDING A DESIGN RISK ASSESSMENT. THE CONTRACTOR SHALL INFORM PELL FRISCHMANN OF ANY PROPOSED VARIANCES TO THE DESIGN.

- STRATEGY NOTES
- *RESIDENTIAL DEVELOPMENT HAS BEEN ASSUMED AT 65%
- *EDUCATION DEVELOPMENT HAS BEEN ASSUMED AT 60%
- **OBAR HAS BEEN CALCULATED AS 4.29l/s/ha
- ***REQUIRED STORAGE ASSESSED FOR THE 100 YEAR + 40% CLIMATE CHANGE EVENT

- KEY
- RED LINE BOUNDARY
- CATCHMENTS
- INDICATIVE ATTENUATION BASIN
- INDICATIVE SURFACE WATER ROUTING
- ATTENUATION SERVING ADJACENT PHASES
- FLOOD ZONE 2
- FLOOD ZONE 3
- OVERLAND FLOOD ROUTING

Summary of Results for 100 year Return Period (+40%)

Storm Event	Rain (mm/hr)	Flooded Volume (m ³)	Discharge Volume (m ³)	Overflow Volume (m ³)	Time-Peak (mins)
15 min Winter	138.460	0.0	323.5	0.0	26
30 min Winter	91.070	0.0	426.4	0.0	40
60 min Winter	56.875	0.0	553.3	0.0	70
120 min Winter	35.332	0.0	688.1	0.0	126
180 min Winter	26.592	0.0	776.5	0.0	184
240 min Winter	21.604	0.0	840.6	0.0	244
360 min Winter	15.907	0.0	926.7	0.0	360
480 min Winter	12.670	0.0	982.1	0.0	474
600 min Winter	10.563	0.0	1021.3	0.0	588

Pell Frischmann		Page 5
5 Manchester Square		
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Model Details

Storage is Online Cover Level (m) 1.500

Tank or Pond Structure

Invert Level (m) 0.000

Depth (m)	Area (m ²)	Depth (m)	Area (m ²)
0.000	865.0	1.500	1497.0

Hydro-Brake® Optimum Outflow Control

Unit Reference	MD-SHE-0131-8300-1200-8300	Sump Available	Yes
Design Head (m)	1.200	Diameter (mm)	131
Design Flow (l/s)	8.3	Invert Level (m)	0.000
Flush-Flo™	Calculated	Minimum Outlet Pipe Diameter (mm)	150
Objective	Minimise upstream storage	Suggested Manhole Diameter (mm)	1200
Application	Surface		

Control Points	Head (m)	Flow (l/s)	Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.200	8.3	Kick-Flo®	0.764	6.7
Flush-Flo™	0.353	8.3	Mean Flow over Head Range	-	7.2

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

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Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Overflow (l/s)	Max Σ Outflow (l/s)	Max Volume (m³)	Status
5 min Winter	0.436	0.436	8.8	0.0	8.8	499.8	O K
0 min Winter	0.559	0.559	8.8	0.0	8.8	655.1	O K
0 min Winter	0.678	0.678	8.8	0.0	8.8	811.1	O K
0 min Winter	0.811	0.811	8.8	0.0	8.8	992.8	O K
0 min Winter	0.889	0.889	8.8	0.0	8.8	1103.9	O K
0 min Winter	0.941	0.941	8.8	0.0	8.8	1177.8	O K
0 min Winter	0.998	0.998	8.8	0.0	8.8	1262.6	O K
0 min Winter	1.024	1.024	8.8	0.0	8.8	1301.5	O K
0 min Winter	1.034	1.034	8.8	0.0	8.8	1316.6	O K

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Pell Frischmann		Page 5
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Model Details

Storage is Online Cover Level (m) 1.500

Tank or Pond Structure

Invert Level (m) 0.000

Depth (m)	Area (m ²)	Depth (m)	Area (m ²)
0.000	1058.0	1.500	1725.0

Hydro-Brake® Optimum Outflow Control

Unit Reference	MD-SHE-0134-8800-1200-8800	Sump Available	Yes
Design Head (m)	1.200	Diameter (mm)	134
Design Flow (l/s)	8.8	Invert Level (m)	0.000
Flush-Flo™	Calculated	Minimum Outlet Pipe Diameter (mm)	150
Objective	Minimise upstream storage	Suggested Manhole Diameter (mm)	1200
Application	Surface		

Control Points	Head (m)	Flow (l/s)	Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.200	8.8	Kick-Flo®	0.769	7.1
Flush-Flo™	0.355	8.8	Mean Flow over Head Range	-	7.6

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

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Summary of Results for 100 year Return Period (+40%)

Storm Event		Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Overflow (l/s)	Max Inflow (l/s)	Max Outflow (l/s)	Max Volume (m³)	Status
720 min	Winter	1.228	1.228	12.5	0.0	12.5	1835.2	Flood	Risk
960 min	Winter	1.207	1.207	12.4	0.0	12.4	1798.7	Flood	Risk
1440 min	Winter	1.135	1.135	12.4	0.0	12.4	1671.1		O K
2160 min	Winter	1.033	1.033	12.4	0.0	12.4	1495.7		O K
2880 min	Winter	0.940	0.940	12.4	0.0	12.4	1340.8		O K
4320 min	Winter	0.758	0.758	12.4	0.0	12.4	1048.7		O K
5760 min	Winter	0.591	0.591	12.4	0.0	12.4	795.7		O K
7200 min	Winter	0.466	0.466	12.4	0.0	12.4	614.3		O K
8640 min	Winter	0.370	0.370	12.4	0.0	12.4	480.2		O K

Storm Event	Rain (mm/hr)	Flooded Volume (m ³)	Discharge Volume (m ³)	Overflow Volume (m ³)	Time-Peak (mins)
720 min Winter	9.077	0.0	1927.5	0.0	704
960 min Winter	7.106	0.0	1890.9	0.0	926
1440 min Winter	4.989	0.0	1807.5	0.0	1332
2160 min Winter	3.489	0.0	2582.2	0.0	1652
2880 min Winter	2.715	0.0	2676.0	0.0	2112
4320 min Winter	1.930	0.0	2841.3	0.0	2992
5760 min Winter	1.531	0.0	3037.3	0.0	3752
7200 min Winter	1.296	0.0	3211.8	0.0	4472
8640 min Winter	1.141	0.0	3389.8	0.0	5112

Pell Frischmann		Page 5
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Model Details

Storage is Online Cover Level (m) 1.500

Tank or Pond Structure

Invert Level (m) 0.000

Depth (m)	Area (m ²)	Depth (m)	Area (m ²)
0.000	1217.0	1.500	1933.0

Hydro-Brake® Optimum Outflow Control

Unit Reference	MD-SHE-0158-1240-1200-1240	Sump Available	Yes
Design Head (m)	1.200	Diameter (mm)	158
Design Flow (l/s)	12.4	Invert Level (m)	0.000
Flush-Flo™	Calculated	Minimum Outlet Pipe Diameter (mm)	225
Objective	Minimise upstream storage	Suggested Manhole Diameter (mm)	1200
Application	Surface		

Control Points	Head (m)	Flow (l/s)	Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.200	12.4	Kick-Flo®	0.792	10.2
Flush-Flo™	0.360	12.4	Mean Flow over Head Range	-	10.7

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

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Summary of Results for 100 year Return Period (+40%)

Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Overflow Volume (m³)	Time-Peak (mins)
15 min Winter	138.460	0.0	620.8	0.0	26
30 min Winter	91.070	0.0	804.7	0.0	41
60 min Winter	56.875	0.0	1064.8	0.0	70
120 min Winter	35.332	0.0	1319.5	0.0	128
180 min Winter	26.592	0.0	1483.4	0.0	186
240 min Winter	21.604	0.0	1597.9	0.0	244
360 min Winter	15.907	0.0	1735.5	0.0	360
480 min Winter	12.670	0.0	1791.7	0.0	476
600 min Winter	10.563	0.0	1793.8	0.0	590

Pell Frischmann		Page 5
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File Catchment C.SRCX	Checked by	
Innovyze	Source Control 2020.1	

Model Details

Storage is Online Cover Level (m) 1.500

Tank or Pond Structure

Invert Level (m) 0.000

Depth (m)	Area (m ²)	Depth (m)	Area (m ²)
0.000	1139.0	1.500	1890.0

Hydro-Brake® Optimum Outflow Control

Unit Reference	MD-SHE-0152-1150-1200-1150	Sump Available	Yes
Design Head (m)	1.200	Diameter (mm)	152
Design Flow (l/s)	11.5	Invert Level (m)	0.000
Flush-Flo™	Calculated	Minimum Outlet Pipe Diameter (mm)	225
Objective	Minimise upstream storage	Suggested Manhole Diameter (mm)	1200
Application	Surface		

Control Points	Head (m)	Flow (l/s)	Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.200	11.5	Kick-Flo®	0.787	9.4
Flush-Flo™	0.358	11.5	Mean Flow over Head Range	-	9.9

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

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<u>Hydro-Brake® Optimum Outflow Control</u> <table border="1"> <thead> <tr> <th>Depth (m)</th> <th>Flow (l/s)</th> <th>Depth (m)</th> <th>Flow (l/s)</th> <th>Depth (m)</th> <th>Flow (l/s)</th> <th>Depth (m)</th> <th>Flow (l/s)</th> <th>Depth (m)</th> <th>Flow (l/s)</th> <th>Depth (m)</th> <th>Flow (l/s)</th> </tr> </thead> <tbody> <tr> <td>0.100</td> <td>5.5</td> <td>0.600</td> <td>11.0</td> <td>1.600</td> <td>13.2</td> <td>2.600</td> <td>16.6</td> <td>5.000</td> <td>22.7</td> <td>7.500</td> <td>27.6</td> </tr> <tr> <td>0.200</td> <td>10.9</td> <td>0.800</td> <td>9.5</td> <td>1.800</td> <td>13.9</td> <td>3.000</td> <td>17.8</td> <td>5.500</td> <td>23.7</td> <td>8.000</td> <td>28.4</td> </tr> <tr> <td>0.300</td> <td>11.4</td> <td>1.000</td> <td>10.6</td> <td>2.000</td> <td>14.6</td> <td>3.500</td> <td>19.1</td> <td>6.000</td> <td>24.8</td> <td>8.500</td> <td>29.3</td> </tr> <tr> <td>0.400</td> <td>11.5</td> <td>1.200</td> <td>11.5</td> <td>2.200</td> <td>15.3</td> <td>4.000</td> <td>20.4</td> <td>6.500</td> <td>25.7</td> <td>9.000</td> <td>30.1</td> </tr> <tr> <td>0.500</td> <td>11.3</td> <td>1.400</td> <td>12.4</td> <td>2.400</td> <td>16.0</td> <td>4.500</td> <td>21.6</td> <td>7.000</td> <td>26.7</td> <td>9.500</td> <td>30.9</td> </tr> </tbody> </table> <u>Weir Overflow Control</u> Discharge Coef 0.544 Width (m) 1.000 Invert Level (m) 1.500								Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	0.100	5.5	0.600	11.0	1.600	13.2	2.600	16.6	5.000	22.7	7.500	27.6	0.200	10.9	0.800	9.5	1.800	13.9	3.000	17.8	5.500	23.7	8.000	28.4	0.300	11.4	1.000	10.6	2.000	14.6	3.500	19.1	6.000	24.8	8.500	29.3	0.400	11.5	1.200	11.5	2.200	15.3	4.000	20.4	6.500	25.7	9.000	30.1	0.500	11.3	1.400	12.4	2.400	16.0	4.500	21.6	7.000	26.7	9.500	30.9
Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)																																																																				
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Pell Frischmann		Page 5
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Model Details

Storage is Online Cover Level (m) 1.500

Tank or Pond Structure

Invert Level (m) 0.000

Depth (m)	Area (m ²)	Depth (m)	Area (m ²)
0.000	1897.0	1.500	2762.0

Hydro-Brake® Optimum Outflow Control

Unit Reference	MD-SHE-0190-1870-1200-1870	Sump Available	Yes
Design Head (m)	1.200	Diameter (mm)	190
Design Flow (l/s)	18.7	Invert Level (m)	0.000
Flush-Flo™	Calculated	Minimum Outlet Pipe Diameter (mm)	225
Objective	Minimise upstream storage	Suggested Manhole Diameter (mm)	1500
Application	Surface		

Control Points	Head (m)	Flow (l/s)	Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.200	18.7	Kick-Flo®	0.823	15.6
Flush-Flo™	0.376	18.6	Mean Flow over Head Range	-	16.0

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

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Pell Frischmann		Page 5
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W1U 3PD		
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File Catchment E.SRCX	Checked by	
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Model Details

Storage is Online Cover Level (m) 1.500

Tank or Pond Structure

Invert Level (m) 0.000

Depth (m)	Area (m ²)	Depth (m)	Area (m ²)
0.000	1220.0	1.500	1878.0

Hydro-Brake® Optimum Outflow Control

Unit Reference	MD-SHE-0131-8300-1200-8300	Sump Available	Yes
Design Head (m)	1.200	Diameter (mm)	131
Design Flow (l/s)	8.3	Invert Level (m)	0.000
Flush-Flo™	Calculated	Minimum Outlet Pipe Diameter (mm)	150
Objective	Minimise upstream storage	Suggested Manhole Diameter (mm)	1200
Application	Surface		

Control Points	Head (m)	Flow (l/s)	Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.200	8.3	Kick-Flo®	0.764	6.7
Flush-Flo™	0.353	8.3	Mean Flow over Head Range	-	7.2

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

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Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)																																																																				
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Appendix H Simple Index Approach Summary

SUMMARY TABLE		DESIGN CONDITIONS			
		1	2	3	4
Land Use Type Pollution Hazard Level Pollution Hazard Indices TSS Metals Hydrocarbons	Roads (excluding low traffic roads, highly frequented lorry approaches to industrial estates, trunk roads/motorways) Medium 0.7 0.6 0.7				
SuDS components proposed		SuDS components can only be assumed to deliver these indices if they follow design guidance with respect to hydraulics and treatment set out in the relevant technical component chapters of the SuDS Manual. See also checklists in Appendix B	Ponds/wetlands should be preceded by an upstream component(s) that trap(s) silt, or designed specifically to retain sediment in a separate zone, easily accessible for maintenance, such that the sediment will not be re-suspended in subsequent events		
Component 1	Pond or wetland				
Component 2	Filter drain (where the trench is not designed as an infiltration component)				
Component 3	None				
SuDS Pollution Mitigation Indices TSS Metals Hydrocarbons	 0.9 0.9 0.7				
Groundwater protection type Groundwater protection Pollution Mitigation Indices TSS Metals Hydrocarbons	None 0 0 0				
Combined Pollution Mitigation Indices TSS Metals Hydrocarbons Acceptability of Pollution Mitigation TSS Metals Hydrocarbons	 0.9 0.9 0.7 Sufficient Sufficient Sufficient	Reference to local planning documents should also be made to identify any additional protection required for sites due to habitat conservation (see Chapter 7 The SuDS design process). The implications of developments on or within close proximity to an area with an environmental designation, such as a Site of Special Scientific Interest (SSSI), should be considered via consultation with relevant conservation bodies such as Natural England			