

Figure 4-2: Baseline scenario climate change peak flood extents

#### 4.2.2 Peak flood depths

Figure 4-3 shows the peak flood depths within the site boundary during the 1% AEP with (+40%) climate change event for the baseline scenario. Peak modelled flood depths of up to 0.99m are predicted to occur within the site during the 100-year (1% AEP) plus climate change event. The deepest areas of flooding are shown to be confined within the drainage channels running from northwest to southeast, along the western boundary of the site and northeast to southwest through the centre of the site.

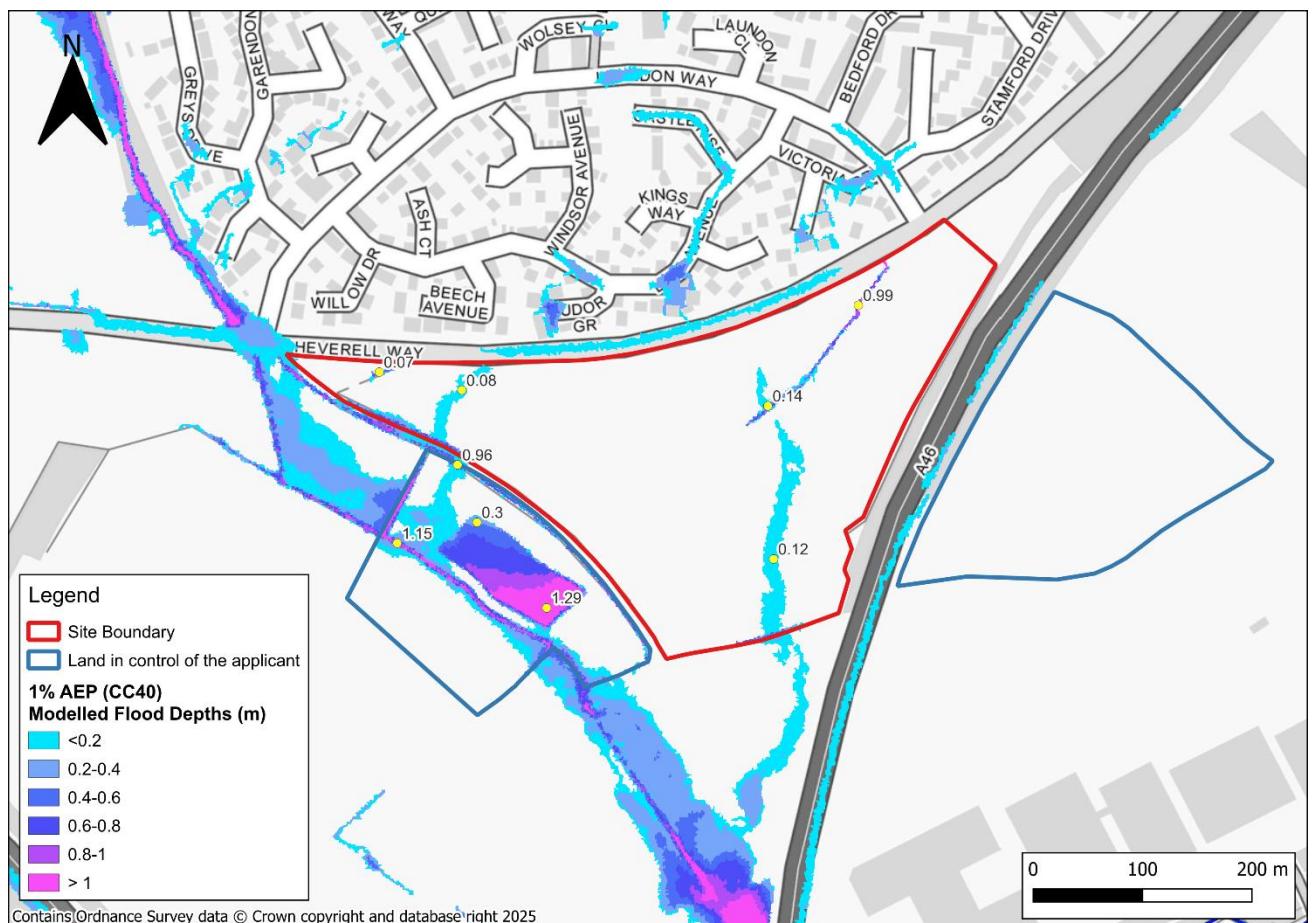


Figure 4-3: Modelled depths in 1% AEP with (+40%) climate change flood event

#### 4.2.3 Peak flood levels

Figure 4-4 shows the peak flood levels within the site boundary during the 1% AEP with (+40%) climate change event for the baseline scenario. Peak modelled flood levels are predicted to range between 84.29m AOD to the north-east and 78.92m AOD towards the southern boundary of the site during the 100-year (1% AEP) plus climate change event.

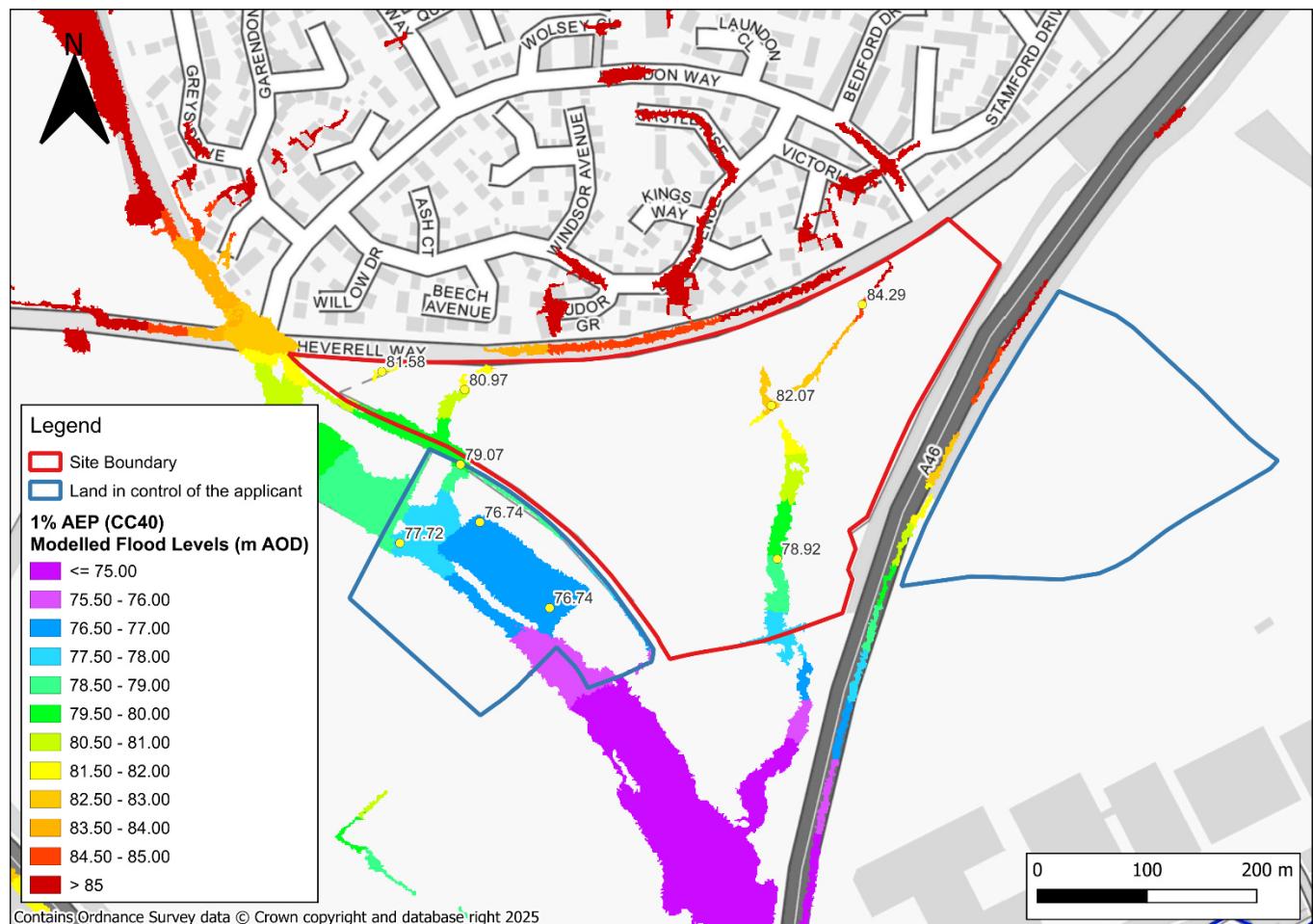


Figure 4-4: Modelled levels in the 1% AEP with (+40) climate change flood event

#### 4.2.4 Hazard to people

The hazard-to-people rating has been mapped using the formula as specified in Defra's FD2320/TR2 "Flood Risk to People". The different hazard categories are shown in Table 4-1 and the hazard classification to the site during the 100-year (1% AEP) plus 40% climate change event is shown in Figure 4-5.

Table 4-1: Defra's FD2320/TR2 "Flood Risks to People" classifications

Flood hazard rating	Degree of Flood Hazard	Description
<0.75	Caution	Caution " <i>Flood zone with shallow flowing water or deep standing water</i> "
0.75 to 1.25	Moderate	Dangerous for some (i.e. Children) " <i>Danger: flood zone with deep or fast flowing water</i> "
1.25 to 2.00	Significant	Dangerous for most " <i>Danger: flood zone with deep fast flowing water</i> "
>2.00	Extreme	Dangerous for all " <i>Extreme danger: flood zone with deep fast flowing water</i> "
Using the hazard equation $HR = d^*(v+0.5) + DF$ Where d = depth of flooding (m) v = velocity of floodwaters (m/sec) DF = debris factor		

Figure 4-5 shows that during the 100-year (1% AEP) plus 40% climate change event. The site is mostly comprised of areas of 'caution', with only these classification areas located on the edge of the eastern and western boundaries. There are a few areas along the southwest of the site that are a 'danger for most' in regions within the confines of the

drainage ditches and the flood relief basin. Along the most south-western drainage ditch, there is a 'danger for all' classification, which stretches across the site boundary.

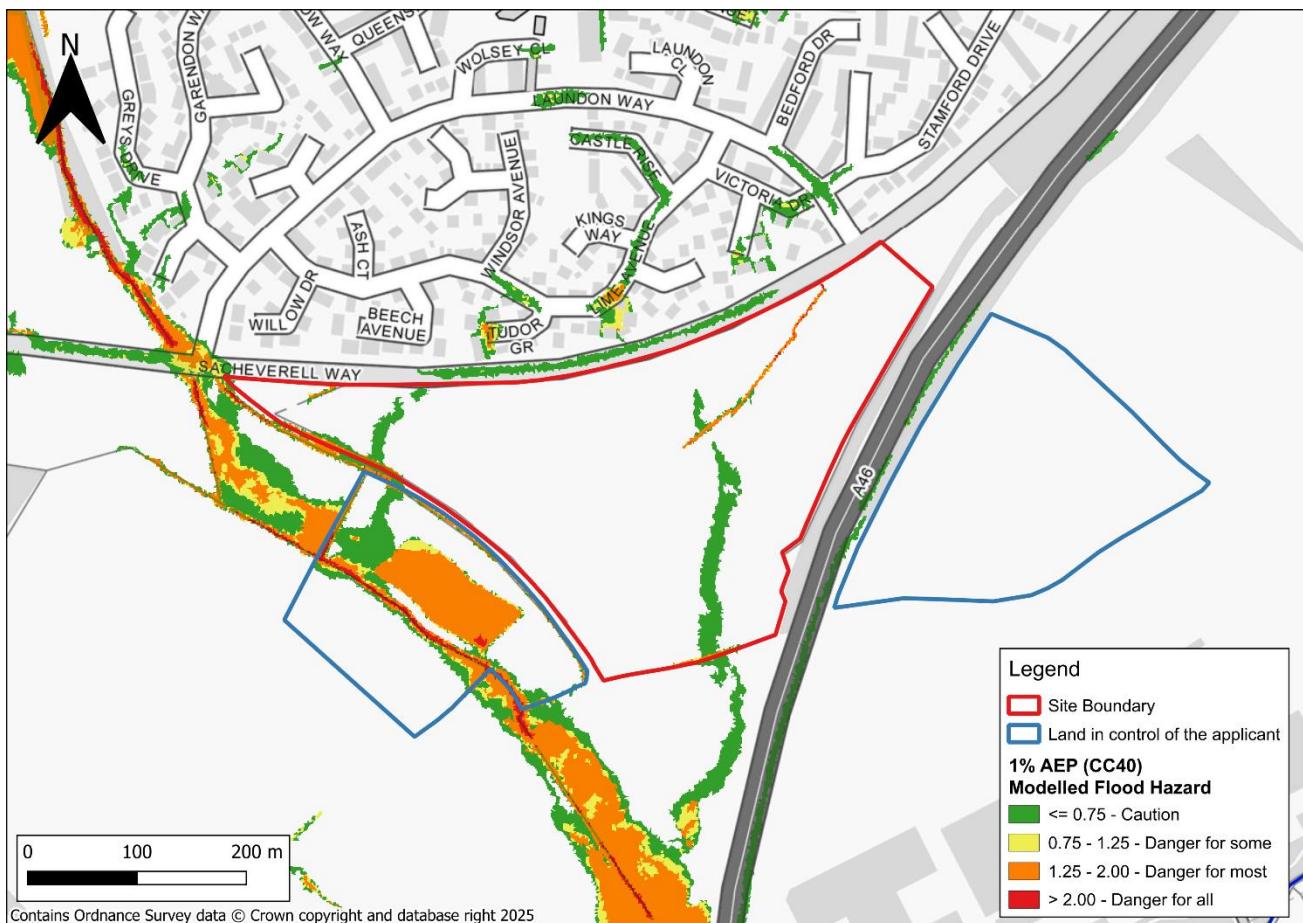


Figure 4-5: Baseline scenario peak flood hazard – 1% AEP plus 40% climate change

#### 4.3 Post-development modelling

A post-development model scenario was developed and was tested for the 1% AEP with (+40%) climate change event. The post-development model scenario is shown in Figure 4-6

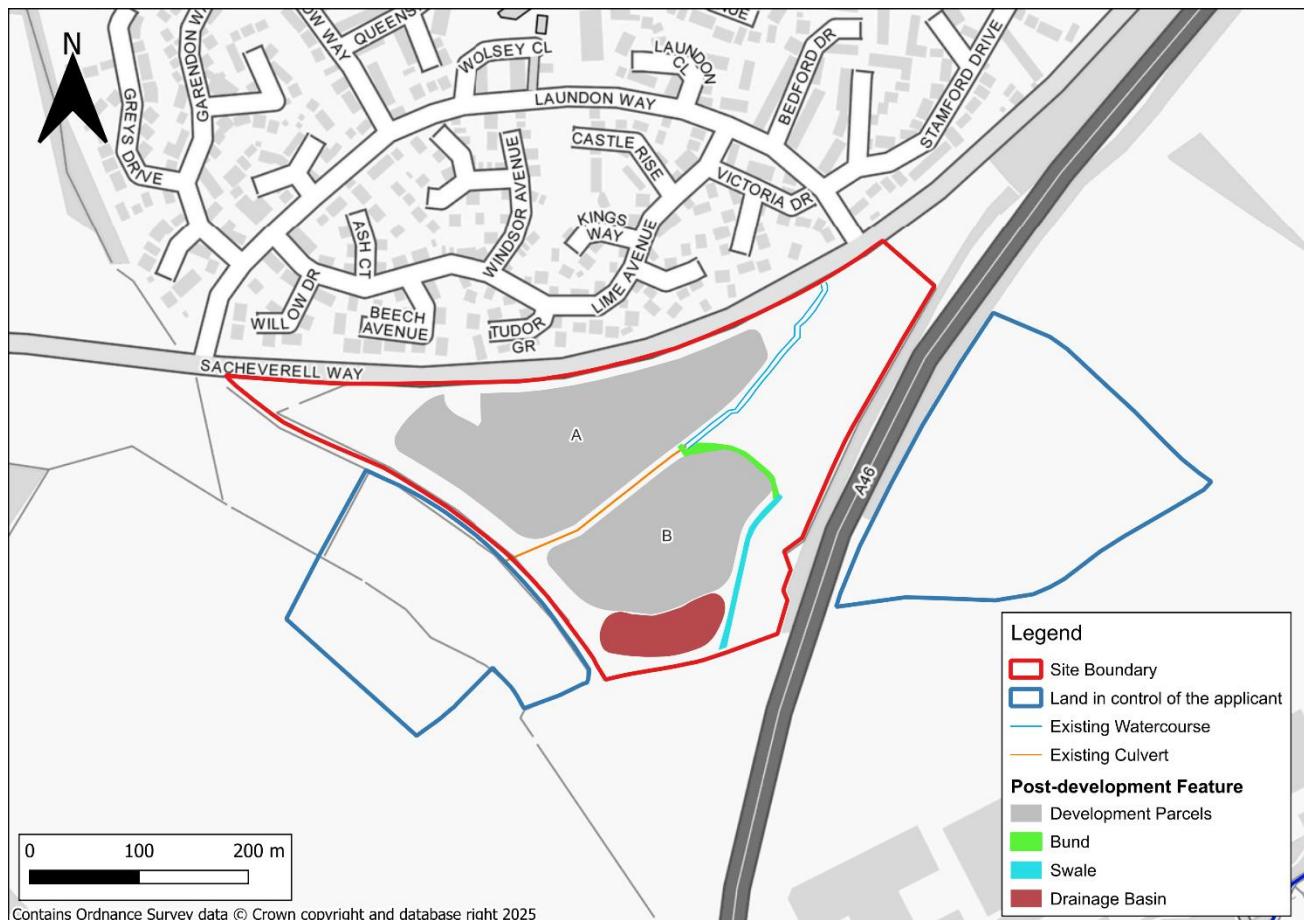


Figure 4-6: Post-Development model scenario

The baseline model was amended by applying proposed ground level changes, including a swale to redirect high flows previously entering the onsite culvert. To facilitate this, ground levels were raised around the culvert inlet, ensuring that when water levels reached 81.6 m AOD, the excess flows could be redirected eastward into a swale designed to channel the water southwards. The existing 900 mm culvert has been reduced to 750 mm to represent the installation of a 750 mm orifice plate at its inlet. This measure aims to lower the proposed downstream flood risk and redirect overland flow through the swale into the drainage ditch along the site's northern boundary.

Further to discussions with the LLFA on the 17th December 2024, it was confirmed that if the existing culvert running through the site was not daylit, the LLFA would not object to the planning application on the basis that the 'status quo if being maintained'.

Indicative development parcels are shown in Figure 4-6. Rainfall inputs were removed from the areas shown to represent the impact of the proposed surface water drainage strategy. This post-development scenario aimed to redirect off-site generated overland flow paths southwards around Development Zone B.



#### 4.3.1 Post-development flood depths

The results of the post-development modelling for the 1% AEP with (+40%) climate change event shows that maximum flood depths within the site vary between 0.11m and 0.41m in Figure 4-7.

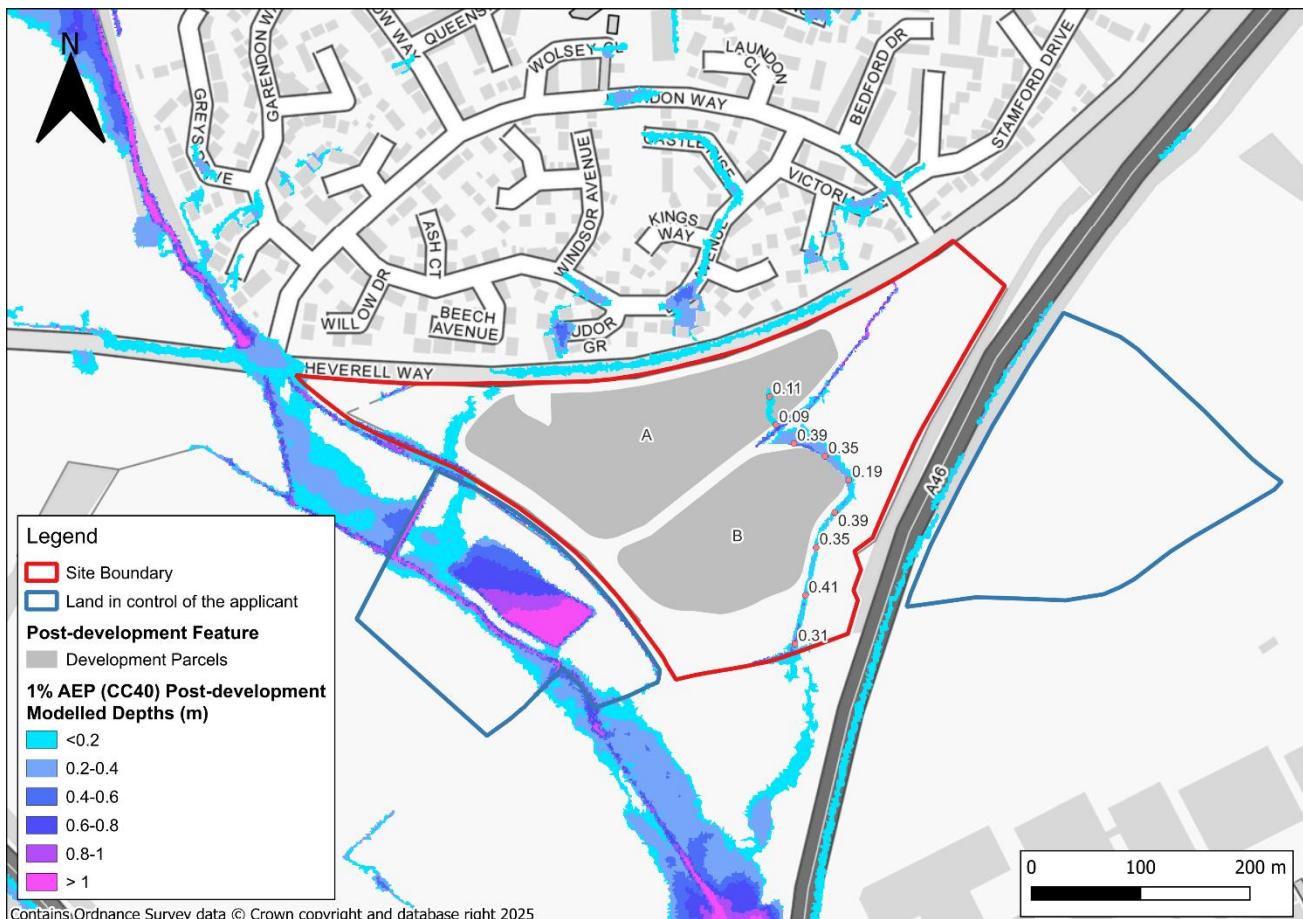


Figure 4-7: Post-development flood depths in the 1% AEP with (+40%) climate change event

#### 4.3.2 Post-development flood levels

Maximum modelled flood levels within the site boundary during the post-development scenario for the 1% AEP with (+40%) climate change event ranged from 82.38m AOD within the central region of the site to 77.9m AOD towards the southern boundary of the site (Figure 4-8).

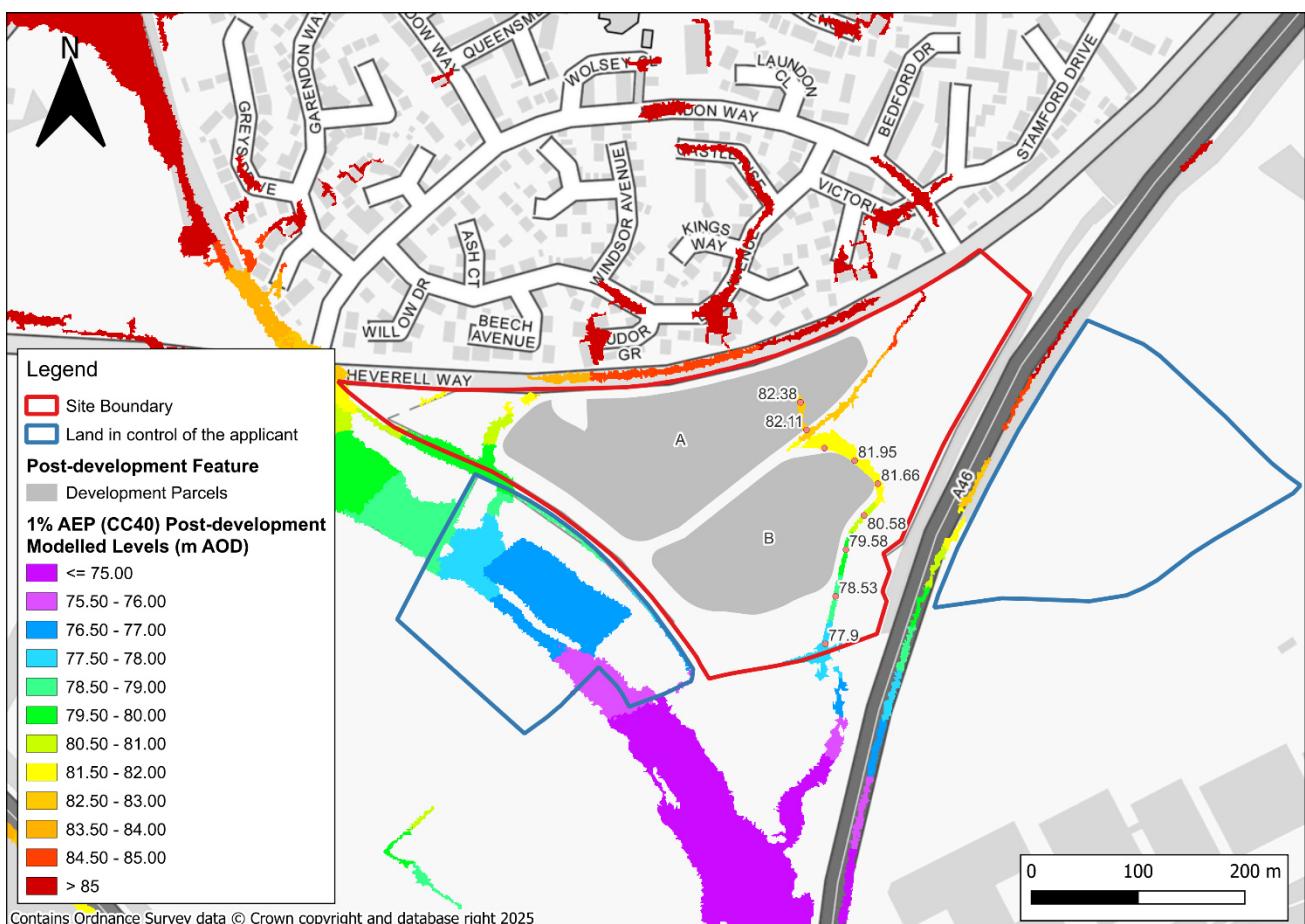


Figure 4-8: Post-development flood levels in the 1% AEP with (+40%) climate change event

#### 4.3.3 Impact of post-development scenario on flood depth

The impact of the post-development model scenario on maximum flood depths has been assessed for the 1% AEP with (+40%) climate change event. The difference between baseline and the post-development maximum flood depths is shown in Figure 4-9.

Overall, the post-development scenario had no significant impact on flood risk at third-party land. The northeast of the site, where the ground modification is located, shows a significant increase in depths of +10cm due to ground level raising in order to divert the overland flow paths. There is a significant reduction south of the modification, which shows the diversion of overland flow paths from this area.

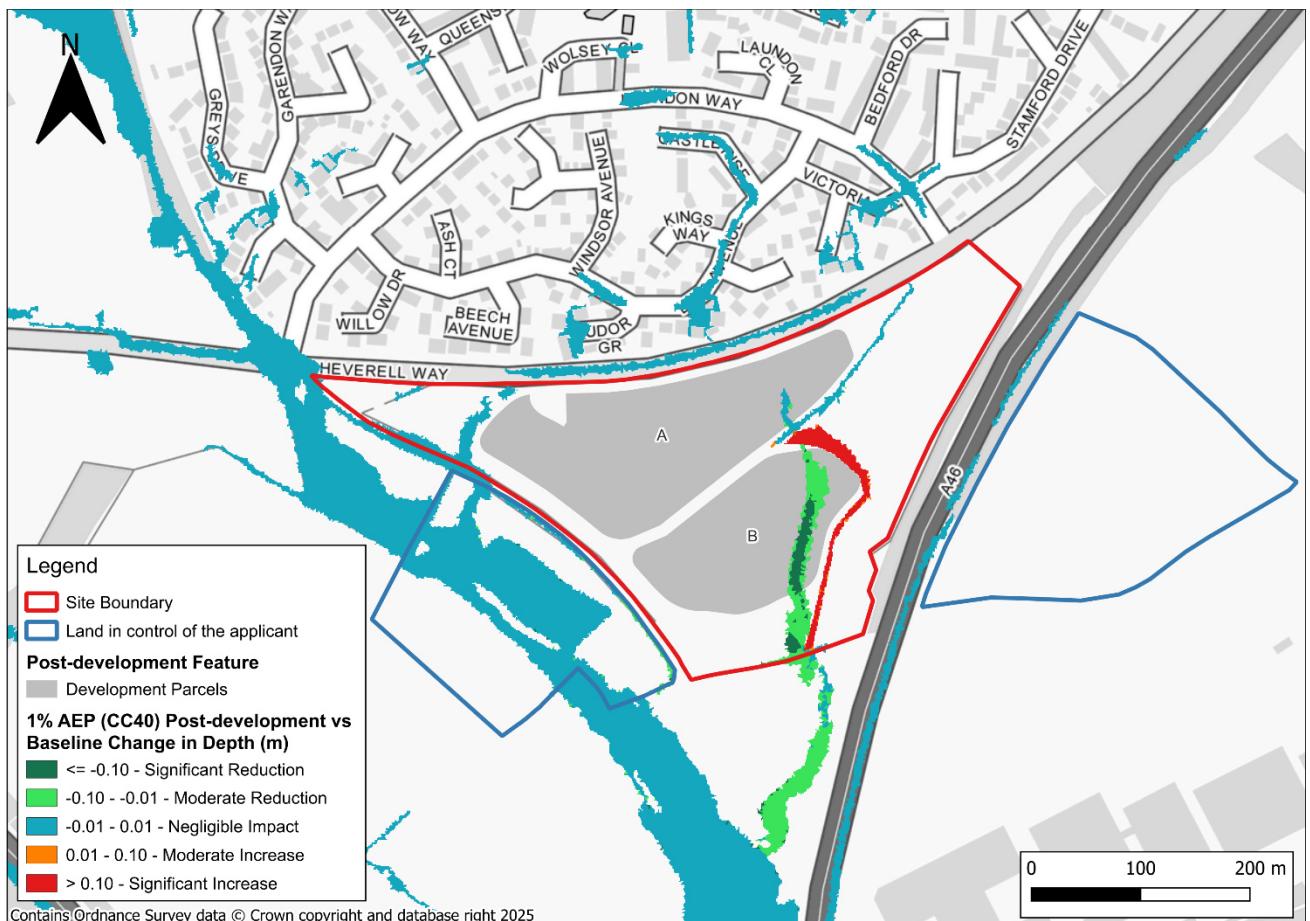


Figure 4-9: Depth comparison between baseline and post-development

#### 4.3.4 Post-development flood hazard

Figure 4-10 shows the post-development hazard rating to the site for the 1% AEP with (+40%) climate change event. The Hazard-to-people rating for the site largely falls within the 'Caution' hazard category, with small, isolated patches along the proposed swale falling into 'Danger for some' and 'Danger for most' hazard category. Overall, there is safe access and egress.

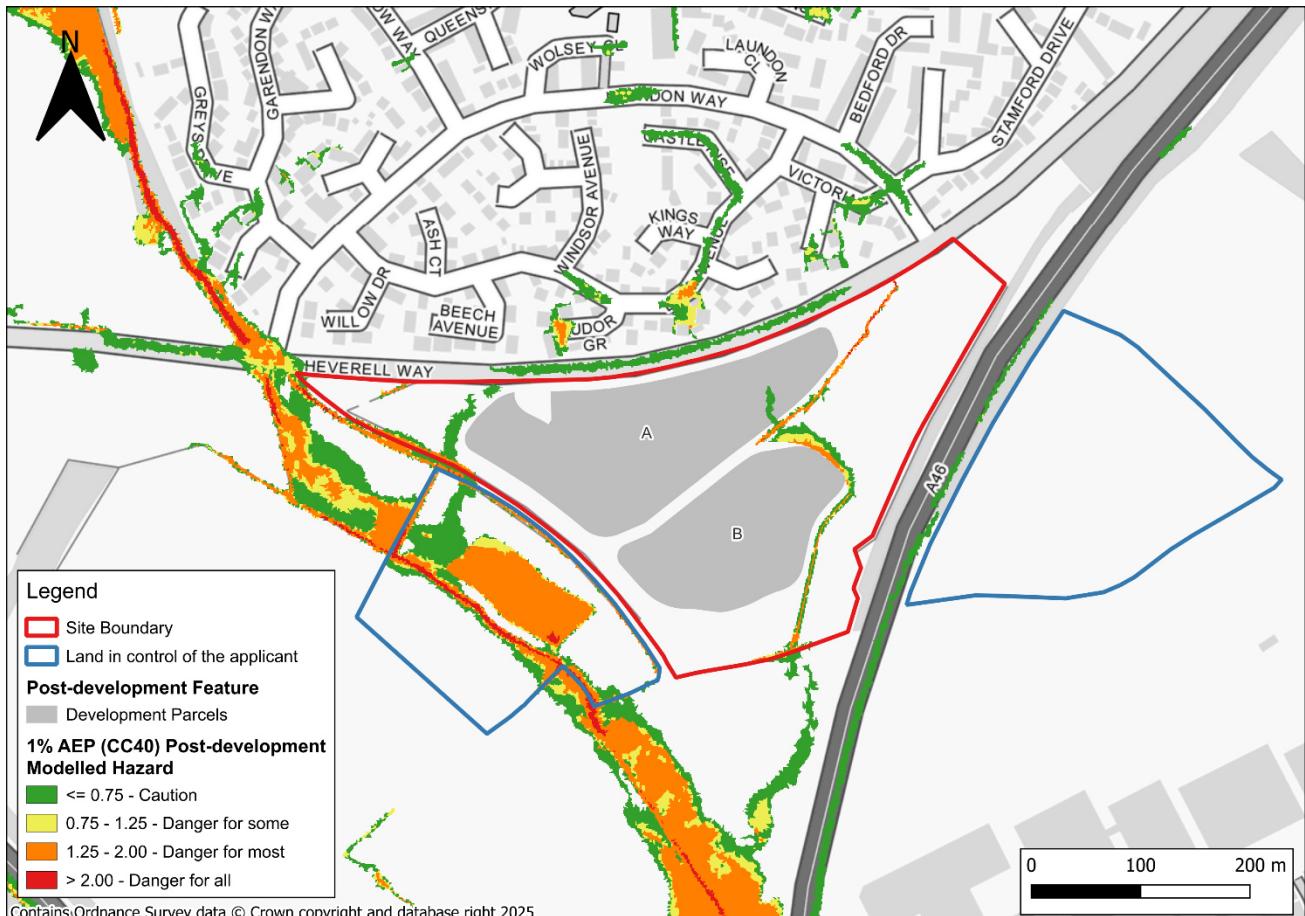


Figure 4-10: Post-development peak flood hazard in the 1% AEP with (+40%) climate change event

# 5 Model assumptions and uncertainties

## 5.1 General assumptions/uncertainties

The representation of any complex system by a model requires a number of assumptions to be made. In the case of the 1D elements of the model the following assumptions have been made:

- Model parameters used, such as roughness and structure coefficients, are representative of the general conditions.
- The units used to represent hydraulic structures within the model represent the situation accurately using the information available at the time of the study.
- A stable numerical solution can be achieved.
- The design hydrology accurately represents flows in the models given there was no flow / level data or historical flood data for the catchment available for calibration of flows in the surface water model.
- The watercourse and culverts around the site have been represented as un-silted using hard bed levels.

In terms of the 2D elements of the model the following assumptions have been made:

- The LIDAR and topographic survey are representative of the land surface and no errors have been introduced through filtering algorithms.
- Model parameters used, such as roughness, are representative of the general conditions.
- The losses applied to rainfall accurately represents the amount of runoff that is lost to infiltration and evaporation.

## 6 Summary and recommendations

### 6.1 Summary

- Bloor Homes East Midlands commissioned JBA Consulting to assess surface water flood risk in relation to a site located off Sacheverall Way, Groby, Leicestershire.
- A 2D InfoWorks-ICM direct runoff hydraulic model was produced to allow the detailed representation of flood depths, flood levels and hazard within the site boundary.
- The model was run for the 3.3% AEP, 3.3% AEP with (+35%) climate change, 1% AEP, 1% AEP with (+40%) climate change, and 0.1% AEP storm events for the 60-minute critical (summer) storm duration.

The baseline model results indicate that:

- Flooding is predicted to occur within the site during all modelled design events.
- Flooding is predicted to occur within the site during the 3.3% AEP with (+35%) climate change and 1% AEP with (+40%) climate change events.
- Peak modelled flood depths of up to 1.29m are predicted to occur within the site during the 100-year (1% AEP) plus climate change event.
- Peak modelled flood levels are predicted to range between 84.29m AOD to the north-east and 78.92m AOD towards the southern boundary of the site during the 100-year (1% AEP) plus climate change event.
- Hazard classification areas of 'caution' extend across the site, with areas of 'Danger for most' to 'Danger for all' located within the confines of the formal defences.
- Sensitivity testing of the model found that within the site boundary the model results are insensitive to changes in modelled roughness, and slightly sensitive to changes in the fixed runoff coefficient (runoff percentage) value.

The post-development model results indicate that:

- Raising the ground levels around the culvert inlet and redirecting the overland flow paths into a swale during the 1% AEP with (+40%) climate change event decreases modelled flood depths to third-party land at the south of the site.
- When comparing depths to the baseline scenario, the northeast of the site, where the ground modification is located, shows a significant increase in depths of +10cm due to ground level raising in order to divert the overland flow paths. There is a significant reduction south of the modification, which shows the diversion of overland flow paths from this area.

## 6.2 Recommendations

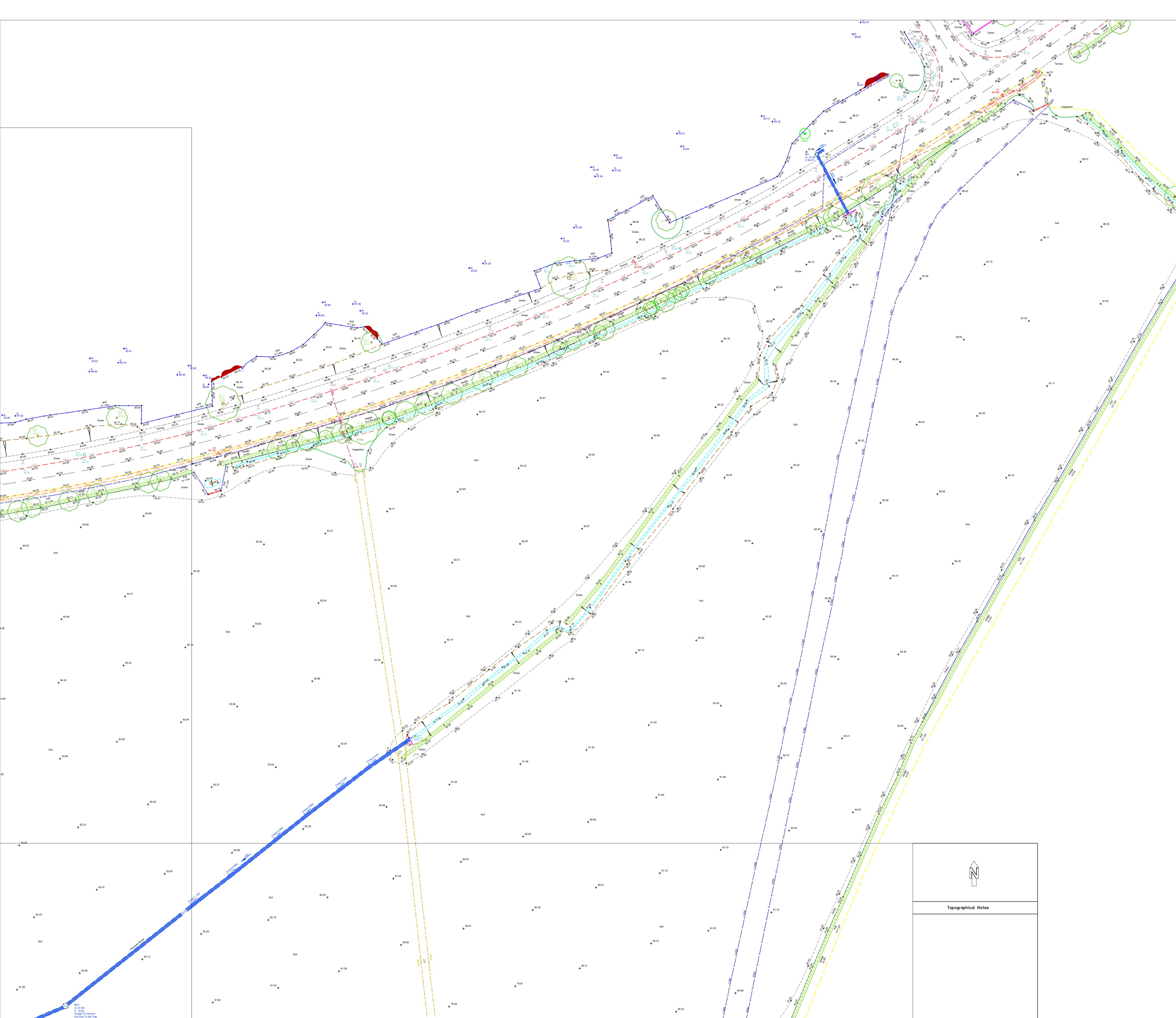
The results of the hydraulic modelling can be used to inform the Flood Risk Assessment for the site. However, it is recommended that the hydraulic modelling is reviewed and validated by the Lead Local Flood Authority, ideally before developing a masterplan or submitting a planning application.

It is recommended that the post-development scenario of the model is altered during the master planning and that Finished Floor Levels (FFLs) are set above the modelled design flood level with freeboard is applied. It is recommended that the new levels be re-simulated in the hydraulic model to understand the impact on flood levels and flood hazard within the site, along with the incorporation of a site surface water drainage strategy.

The model results have been prepared for the purpose of quantifying surface water flood risk at the site and surrounding area. If the results are intended to be used for surface water flood risk mapping of the wider catchment, further model refinement is recommended. Additionally, if further data is made available it is recommended that the model is updated to improve the representation of surface water flood risk parameters.

# Appendices

## A Culvert Survey



AMBER UTILITIES LTD

Bloor Homes

LOCATION:  
Bacheverell Way  
Bachbury

Leicester

## Utility Survey

DF MV  
S NO: U24-11906 SURVEY DATE: October

1:500 @A1



## AMBER UTILITIES LTD

Bloor Homes

cheverell Way  
obby  
chester

**TITLE:** Utility Survey

BY: DF	DRAWN BY: MV	APPROVED BY:
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U24-11906	October
1:500 @A1	SHEET NO: 2 OF 4

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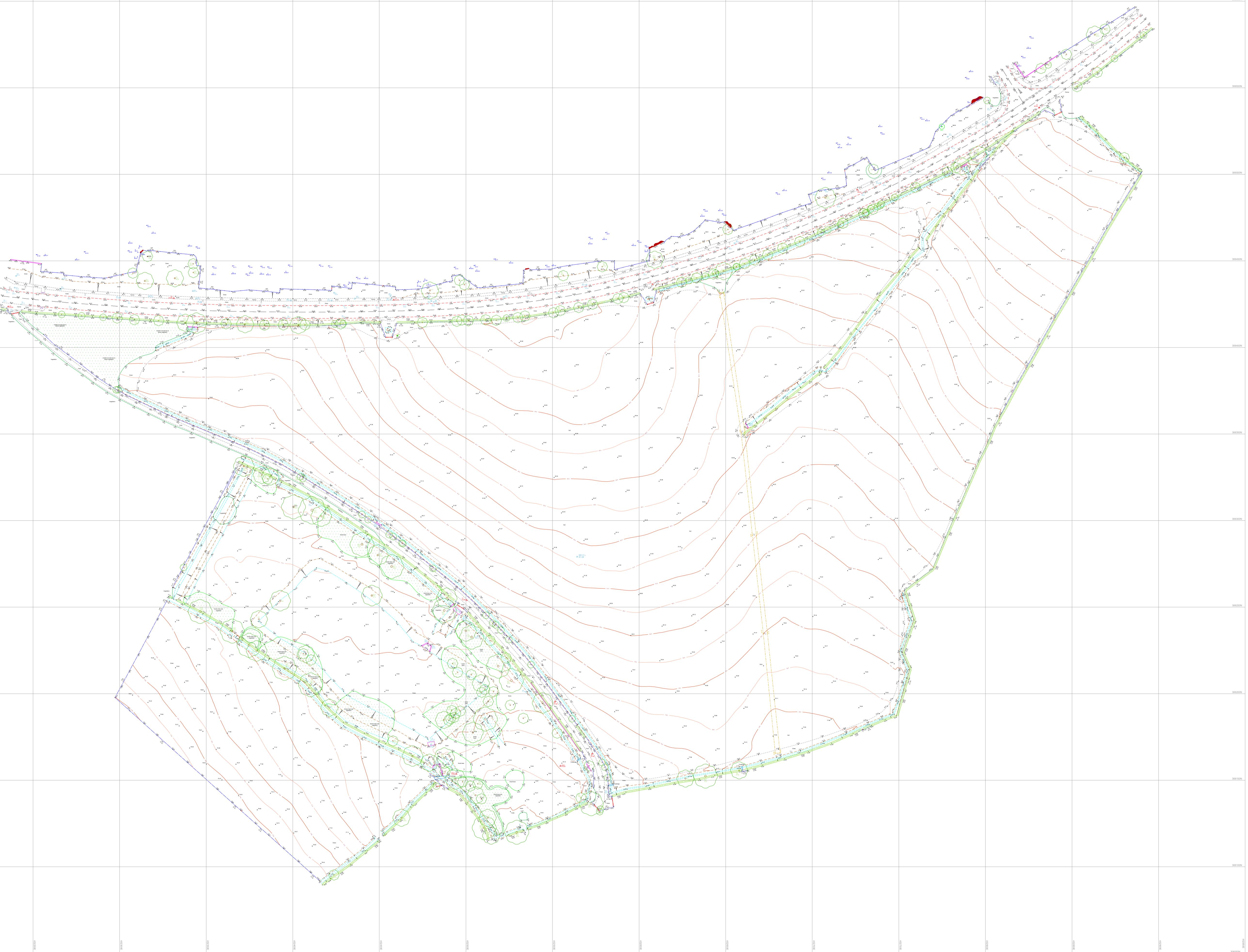




## **B Topographic Survey**

### **B.1 Topographic Survey drawing**

## urban contours



## B.2 Topographic Survey data

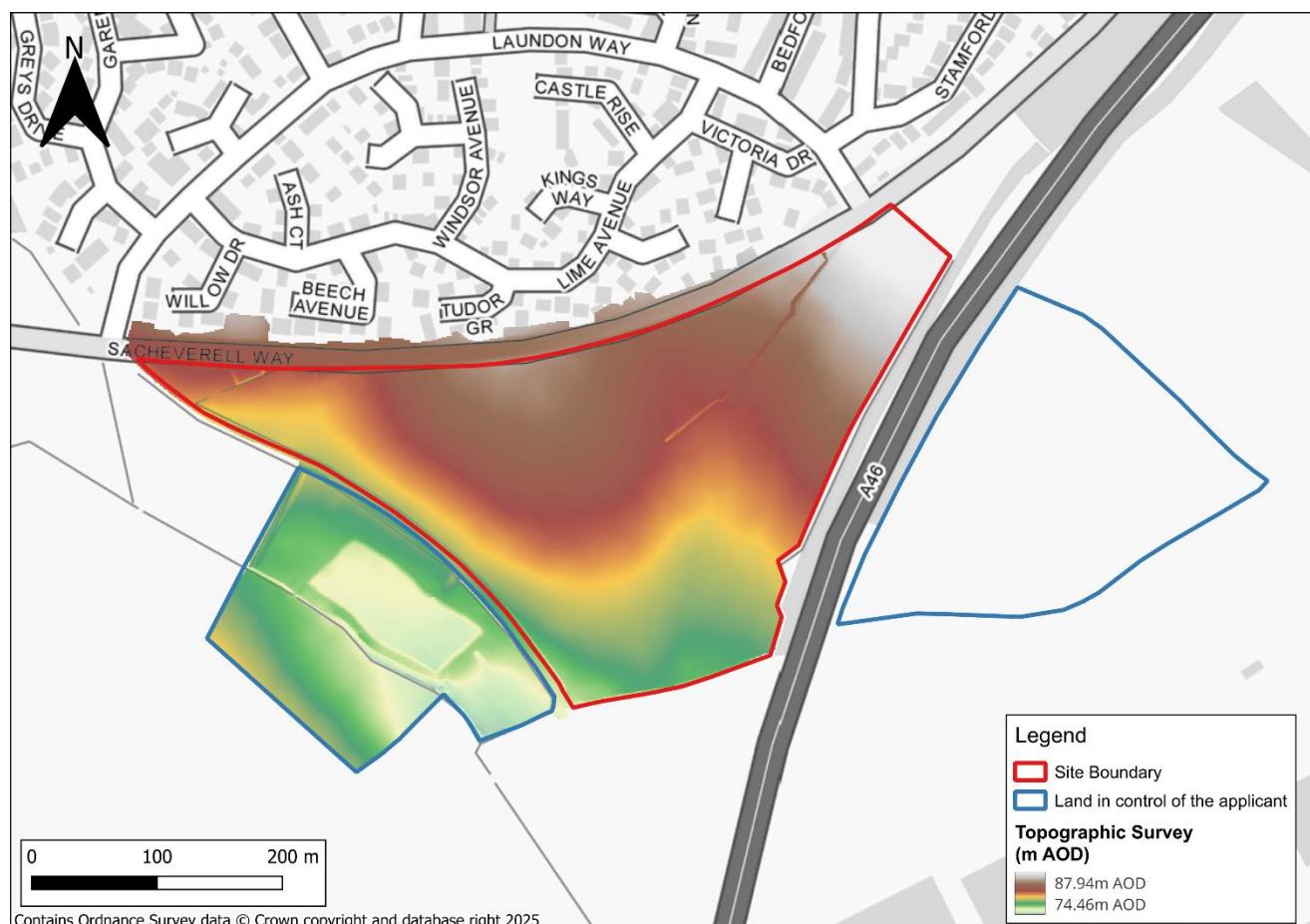


Figure B-1: Topographic Survey

### B.3 Quality assessment

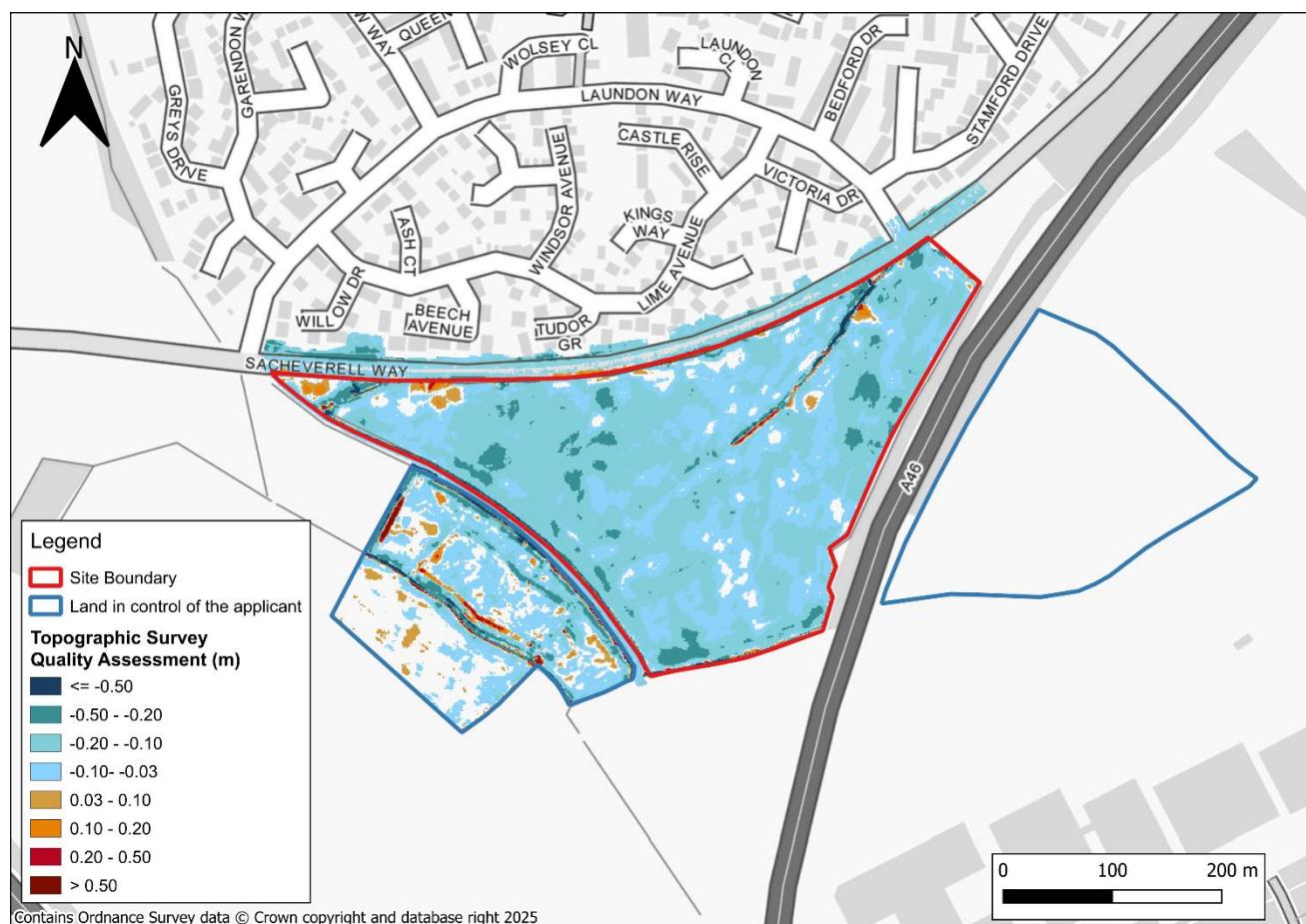
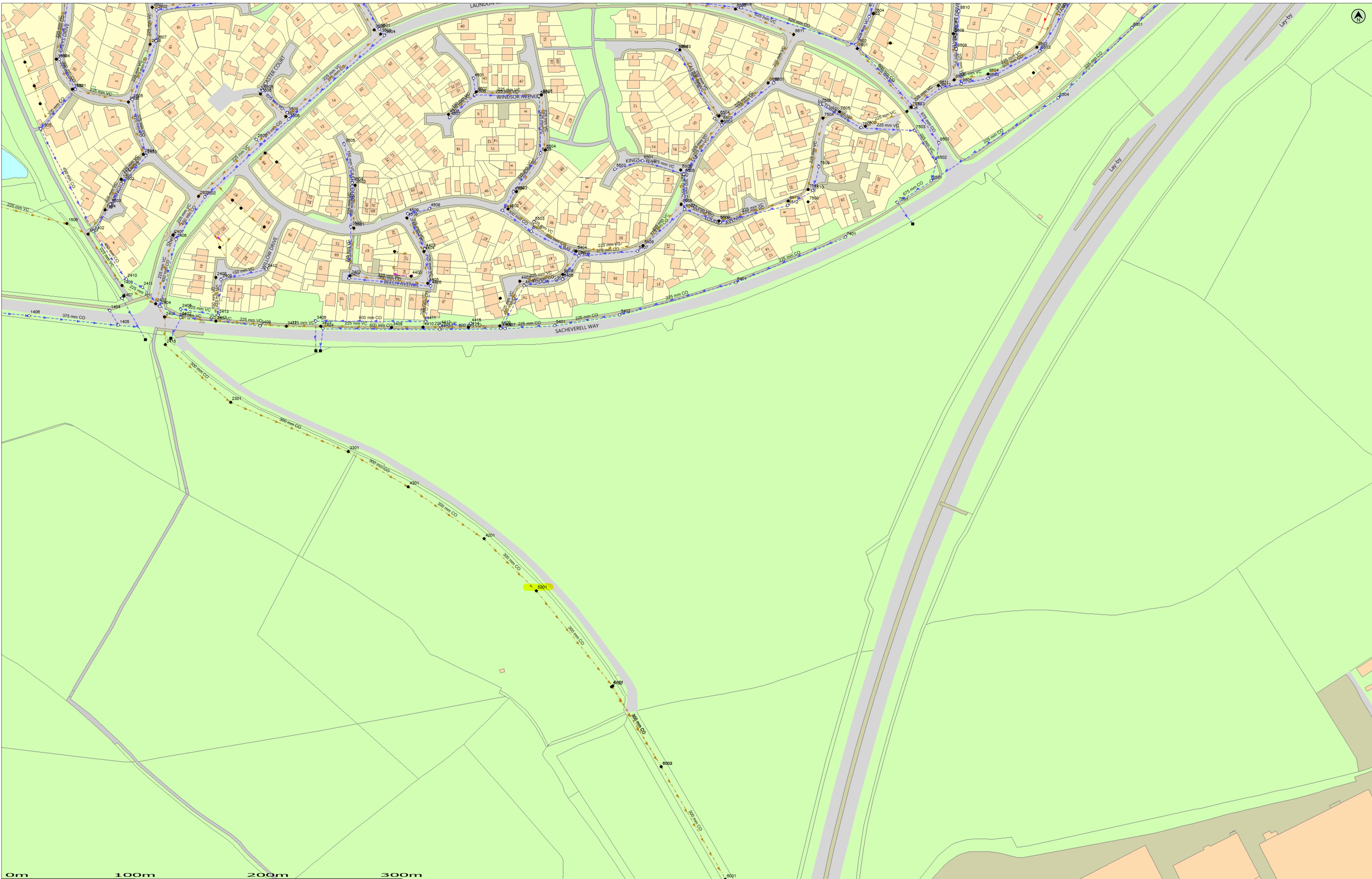


Figure B-2: Topographic Survey Quality Assessment

## **C Severn Trent Water sewer maps**



**GENERAL CONDITIONS AND PRECAUTIONS TO BE TAKEN WHEN CARRYING OUT WORK ADJACENT TO SEVERN TRENT WATER'S APPARATUS**

Please ensure that a copy of these conditions is passed to your representative and/or your contractor on site. If any damage is caused to Severn Trent Water Limited (STW) apparatus (defined below), the person, contractor or subcontractor responsible must inform STW immediately on:  
**0800 783 4444 (24 hours)**

- a) These general conditions and precautions apply to the public sewerage, water distribution and cables in ducts including (but not limited to) sewers which are the subject of an Agreement under Section 104 of the Water Industry Act 1991 (a legal agreement between a developer and STW, where a developer agrees to build sewers to an agreed standard, which STW will then adopt); mains installed in accordance with an agreement for the self-construction of water mains entered into with STW and the assets described at condition b) of these general conditions and precautions. Such apparatus is referred to as "STW Apparatus" in these general conditions and precautions.
- b) Please be aware that due to The Private Sewers Transfer Regulations June 2011, the number of public sewers has increased, but many of these are not shown on the public sewer record. However, some idea of their positions may be obtained from the position of inspection covers and their existence must be anticipated.
- c) On request, STW will issue a copy of the plan showing the approximate locations of STW Apparatus although in certain instances a charge will be made. The position of private drains, private sewers and water service pipes to properties are not normally shown but their presence must be anticipated. This plan and the information supplied with it is furnished as a general guide only and STW does not guarantee its accuracy.
- d) STW does not update these plans on a regular basis. Therefore the position and depth of STW Apparatus may change and this plan is issued subject to any such change. Before any works are carried out, you should confirm whether any changes to the plan have been made since it was issued.
- e) The plan must not be relied upon in the event of excavations or other works in the vicinity of STW Apparatus. It is your responsibility to ascertain the precise location of any STW Apparatus prior to undertaking any development or other works (including but not limited to excavations).
- f) No person or company shall be relieved from liability for loss and/or damage caused to STW Apparatus by reason of the actual position and/or depths of STW Apparatus being different from those shown on the plan.

In order to achieve safe working conditions adjacent to any STW Apparatus the following should be observed:

1. All STW Apparatus should be located by hand digging prior to the use of mechanical excavators.
2. All information set out in any plans received from us, or given by our staff at the site of the works, about the position and depth of the mains, is approximate. Every possible precaution should be taken to avoid damage to STW Apparatus. You or your contractor must ensure the safety of STW Apparatus and will be responsible for the cost of repairing any loss and/or damage caused (including without limitation replacement parts).
3. Water mains are normally laid at a depth of 900mm. No records are kept of customer service pipes which are normally laid at a depth of 750mm; but some idea of their positions may be obtained from the position of stop tap covers and their existence must be anticipated.
4. During construction work, where heavy plant will cross the line of STW Apparatus, specific crossing points must be agreed with STW and suitably reinforced where required. These crossing points should be clearly marked and crossing of the line of STW Apparatus at other locations must be prevented.
5. Where it is proposed to carry out piling or boring within 20 metres of any STW Apparatus, STW should be consulted to enable any affected STW Apparatus to be surveyed prior to the works commencing.
6. Where excavation of trenches adjacent to any STW Apparatus affects its support, the STW Apparatus must be supported to the satisfaction of STW. Water mains and some sewers are pressurised and can fail if excavation removes support to thrust blocks to bends and other fittings.
7. Where a trench is excavated crossing or parallel to the line of any STW Apparatus, the backfill should be adequately compacted to prevent any settlement which could subsequently cause damage to the STW Apparatus. In special cases, it may be necessary to provide permanent support to STW Apparatus which has been exposed over a length of the excavation before backfilling and reinstatement is carried out. There should be no concrete backfill in contact with the STW Apparatus.
8. No other apparatus should be laid along the line of STW Apparatus irrespective of clearance. Above ground apparatus must not be located within a minimum of 3 metres either side of the centre line of STW Apparatus for smaller sized pipes and 6 metres either side for larger sized pipes without prior approval. No manhole or chamber shall be built over or around any STW Apparatus.
9. A minimum radial clearance of 300 millimetres should be allowed between any plant or equipment being installed and existing STW Apparatus. We reserve the right to increase this distance where strategic assets are affected.
10. Where any STW Apparatus coated with a special wrapping is damaged, even to a minor extent, STW must be notified and the trench left open until the damage has been inspected and the necessary repairs have been carried out. In the case of any material damage to any STW Apparatus causing leakage, weakening of the mechanical strength of the pipe or corrosion-protection damage, the necessary remedial work will be recharged to you.
11. It may be necessary to adjust the finished level of any surface boxes which may fall within your proposed construction. Please ensure that these are not damaged, buried or otherwise rendered inaccessible as a result of the works and that all stop taps, valves, hydrants, etc. remain accessible and operable. Minor reduction in existing levels may result in conflict with STW Apparatus such as valve spindles or tops of hydrants housed under the surface boxes. Checks should be made during site investigations to ascertain the level of such STW Apparatus in order to determine any necessary alterations in advance of the works.
12. With regard to any proposed resurfacing works, you are required to contact STW on the number given above to arrange a site inspection to establish the condition of any STW Apparatus in the nature of surface boxes or manhole covers and frames affected by the works. STW will then advise on any measures to be taken, in the event of this a proportionate charge will be made.
13. You are advised that STW will not agree to either the erection of posts, directly over or within 1.0 metre of valves and hydrants.
14. No explosives are to be used in the vicinity of any STW Apparatus without prior consultation with STW.

**TREE PLANTING RESTRICTIONS**

There are many problems with the location of trees adjacent to sewers, water mains and other STW Apparatus and these can lead to the loss of trees and hence amenity to the area which many people may have become used to. It is best if the problem is not created in the first place. Set out below are the recommendations for tree planting in close proximity to public sewers, water mains and other STW Apparatus.

15. Please ensure that, in relation to STW Apparatus, the mature root systems and canopies of any tree planted do not and will not encroach within the recommended distances specified in the notes below.
16. Both Poplar and Willow trees have extensive root systems and should not be planted within 12 metres of a sewer, water main or other STW Apparatus.
17. The following trees and those of similar size, be they deciduous or evergreen, should not be planted within 6 metres of a sewer, water main or other STW Apparatus. E.g. Ash, Beech, Birch, most Conifers, Elm, Horse Chestnut, Lime, Oak, Sycamore, Apple and Pear. Asset Protection Statements Updated May 2014
18. STW personnel require a clear path to conduct surveys etc. No shrubs or bushes should be planted within 2 metre of the centre line of a sewer, water main or other STW Apparatus.
19. In certain circumstances, both STW and landowners may wish to plant shrubs/bushes in close proximity to a sewer, water main or other STW Apparatus for screening purposes. The following are shallow rooting and are suitable for this purpose: Blackthorn, Broom, Cotoneaster, Elder, Hazel, Laurel, Privet, Quickthorn, Snowberry, and most ornamental flowering shrubs.

Manhole Reference	Liquid Type	Cover Level	Invert Level	Depth to Invert
	F	0	0	
	F	0	0	
	F	0	0	
	F	0	0	
	F	0	0	
	F	0	0	
	F	0	0	
	F	0	0	
	F	0	0	
	F	0	0	
	F	0	0	
1401	F	82.86	81.3	1.56
1403	F	82.12	80.81	1.31
1501	F	84.41	82.34	2.07
1504	F	83.58	81.8	1.78
1506	F	83.29	81.37	1.92
1602	F	85.3	83.64	1.66
1603	F	85.44	83.85	1.59
2301	F	80.94	78.26	2.68
2401	F	82.11	79.37	2.74
2403	F	82.34	80.53	1.81
2407	F	84.64	82.06	2.58
2408	F	85.54	83.52	2.02
2412	F	82.46	79.75	2.71
2415	F	81.5	79.18	2.32
2501	F	86.47	83.83	2.64
2504	F	85.19	82.8	2.39
2506	F	85.53	83.28	2.25
2601	F	88.86	86.93	1.93
2608	F	87.57	85.76	1.81
3301	F	79.68	77.3	2.38
3402	F	84.99	82.92	2.07
3403	F	82.97	80.43	2.54
3408	F	83.35	81.13	2.22
3411	F	82.77	80.25	2.52
3501	F	87.83	85.83	2
3503	F	89.48	87.52	1.96
3507	F	90.45	87.85	2.6
3602	F	93.75	90.92	2.83
3603	F	93.77	90.79	2.98
3606	F	90.03	88.13	1.9
4201	F	78.54	76.17	2.37
4301	F	79.02	76.87	2.15
4400	F	0	0	
4402	F	84.91	82.28	2.63
4404	F	85.85	83.96	1.89
4405	F	84.71	82.28	2.43
4409	F	83.88	81.92	1.96
4410	F	83.47	81.36	2.11
4414	F	83.7	81.69	2.01
4502	F	86.01	83.72	2.29
4503	F	86.18	84.15	2.03
4506	F	90.14	88.39	1.75
4510	F	87.12	84.8	2.32
4602	F	89.89	87.97	1.92
5101	F	76.55	74.66	1.89
5102	F	76.55	0	0
5201	F	77.84	75.67	2.17
5404	F	85.82	82.72	3.1
5406	F	85.84	82.58	3.26
5408	F	85.48	83.13	2.35
5504	F	87.21	85.11	2.1
5605	F	88.79	86.64	2.15
6001	F	0	0	
6002	F	75.86	0	0
6003	F	75.86	0	0
6502	F	88.95	86.54	2.41
6504	F	88.86	86.68	2.18
6506	F	87.09	84.74	2.35
6508	F	85.94	83.5	2.44
6509	F	85.92	83.81	2.11
6511	F	87.25	84.76	2.49
6602	F	89.72	87.49	2.23
6603	F	89.29	87.54	1.75
6606	F	91.26	88.99	2.27
6611	F	90.4	88.73	1.67
7500	F	0	0	
7504	F	88.59	86.12	2.47
7505	F	88.73	86.22	2.51
7508	F	88.37	86.39	1.98
7511	F	86.82	84.94	1.88
7512	F	88.82	86.63	2.19
7514	F	88.87	86.55	2.32
7601	F	89.53	87.05	2.48
7604	F	90.92	87.69	3.23
8602	F	91.48	89.11	2.37
8604	F	90.71	88.85	1.86
8607	F	90.28	88.12	2.16
8609	F	90.96	89.04	1.92
8611	F	89.74	86.87	2.87
1402	S	83.15	81.69	1.46
1404	S	82.81	81.14	1.67
1405	S	82.62	80.88	1.74
1406	S	84.72	82.87	1.85
1407	S	80.42	78.6	1.82
1502	S	84.43	82.9	1.53
1503	S	83.77	82.38	1.39
1505	S	85.14	83.33	1.81
1601	S	85.33	84.01	1.32
1604	S	85.5	84.24	1.26
2402	S	82.26	80.41	1.85
2404	S	82.25	80.95	1.3
2406	S	84.46	82.38	2.08
2409	S	85.53	83.82	1.71
2410	S	82.56	81.28	1.28
2411	S	82.84	81.19	1.65
2413	S	84.42	81.44	2.98
2502	S	86.64	84.34	2.3
2503	S	85.29	83.42	1.87
2505	S	85.65	83.76	1.89
2507	S	89.22	87.11	2.11
2602	S	88.77	87.43	1.34
2607	S	87.58	86.36	1.22
3401	S	84.96	83.08	1.88
3404	S	82.93	80.95	1.98
3406	S	83.96	80.77	3.19
3409	S	82.58	81.09	1.49
3410	S	86.25	84.75	1.5
3502	S	87.87	86.33	1.54
3504	S	89.54	88.05	1.49
3505	S	91.23	89.71	1.52
3506	S	90.41	88.2	2.21
3508	S	90.47	88.57	1.9
3601	S	93.71	91.17	2.54
3604	S	93.79	91.09	2.7
3605	S	90	88.65	1.35
4401	S	85.06	82.46	2.6
4403	S	85.88	84.37	1.51
4406	S	84.69	82.6	2.09
4407	S	83.87	81.94	1.93
4408	S	83.84	81.89	1.95
4411	S	83.67	82.02	1.65
4412	S	83.47	81.59	1.88
4413	S	83.6	82.4	1.2
4415	S	83.75	82.7	1.05
4501	S	86.03	84.05	1.98
4504	S	86.18	84.53	1.65
4507	S	90.13	88.66	1.47
4508	S	86.85	85.46	1.39
4509	S	87.13	85.21	1.92
4601	S	90.42	88.87	1.55
4606	S	89.85	88.23	1.62
5401	S	84.1	82.47	1.63
5402	S	84.42	82.78	1.64
5403	S	85.81	83.24	2.57
5405	S	85.85	82.99	2.86
5407	S	85.43	83.59	1.84
5409	S	85.71	83.64	2.07
5501	S	87.01	85.04	1.97
5502	S	86.66	85.36	1.3
5503	S	85.93	83.78	2.15
5508	S	87.17	85.73	1.44
5604	S	88.81	87.11	1.7
6401	S	84.99	83.31	1.68
6501	S	88.93	86.89	2.04
6503	S	88.99	87.13	1.86
6505	S	86.97	84.23	2.74
6507	S	85.87	83.9	1.97
6510	S	85.85	84.17	1.68
6512	S	87.08	85.11	1.97
6601	S	89.68	87.96	1.72
6604	S	89.32	87.8	1.52
6605	S	91.17	88.14	3.03
7501	S	87.62	86.04	1.58
7503	S	88.97	86.96	2.01
7506	S	88.97	87.66	



## D Sensitivity Analysis

### D.1 Sensitivity to roughness

Figure D-1 shows the impact on flood extents within the site when the Manning's 'n' roughness values are increased and decreased by 20%. The model results indicate a slight increase in flood extent within the site when the roughness was decreased by 20%, while a 20% increase in roughness remains the same as the baseline extent. It shows that within the development site, the model is not sensitive to roughness changes.

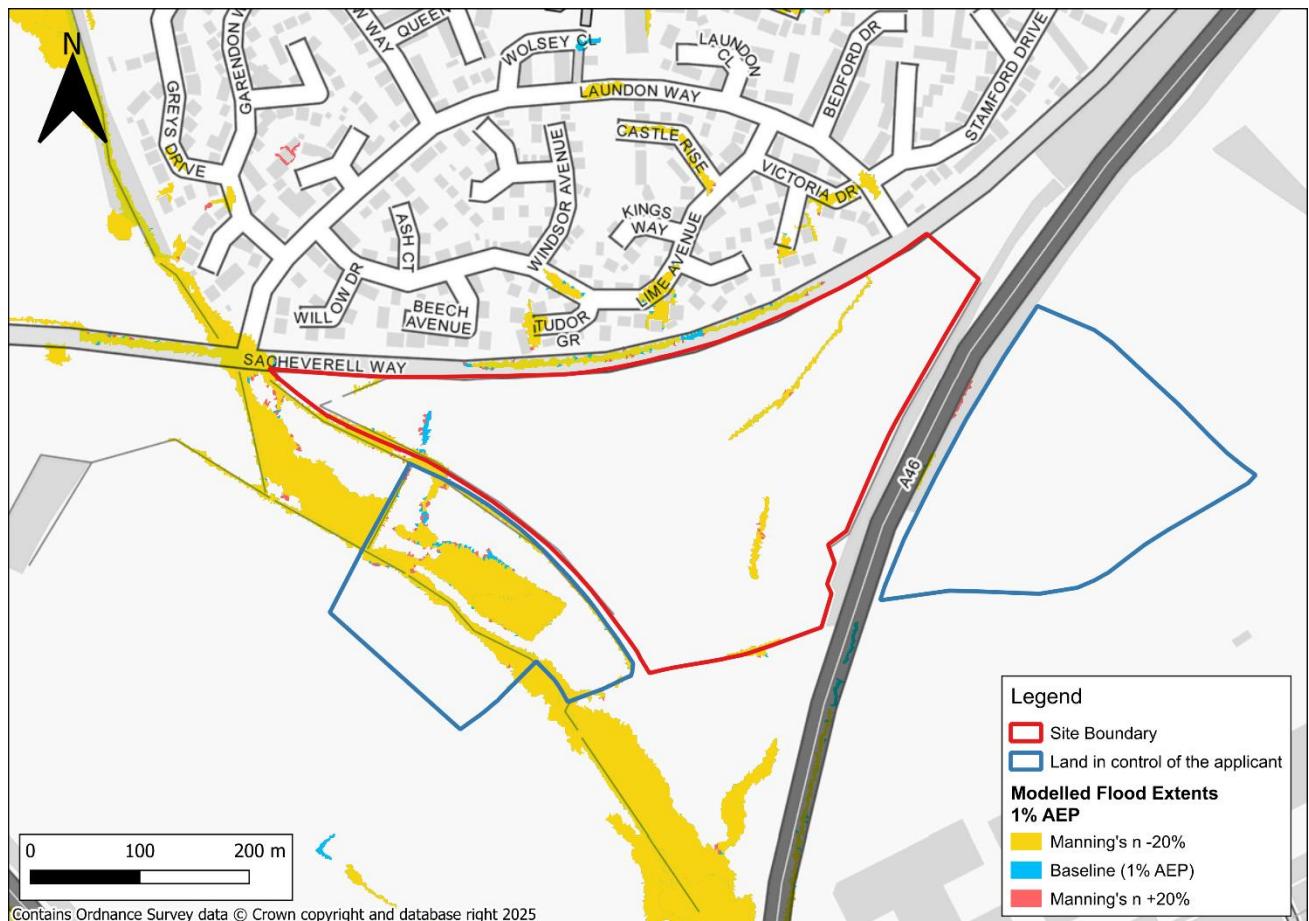


Figure D-1: Sensitivity to changes in roughness coefficients

## D.2 Sensitivity to fixed runoff coefficient (FRC)

Figure D- 2 shows the impact on flood extents within the site when the fixed runoff coefficient values are increased and decreased by 20%. The model results indicate a minor increase in flood extent within the site when the fixed runoff coefficient was increased by 20%, while a 20% reduction in the fixed runoff coefficient is predicted to result in a corresponding reduction in flood extent. This suggests that the model is slightly sensitive to changes in fixed runoff coefficient values in this location.

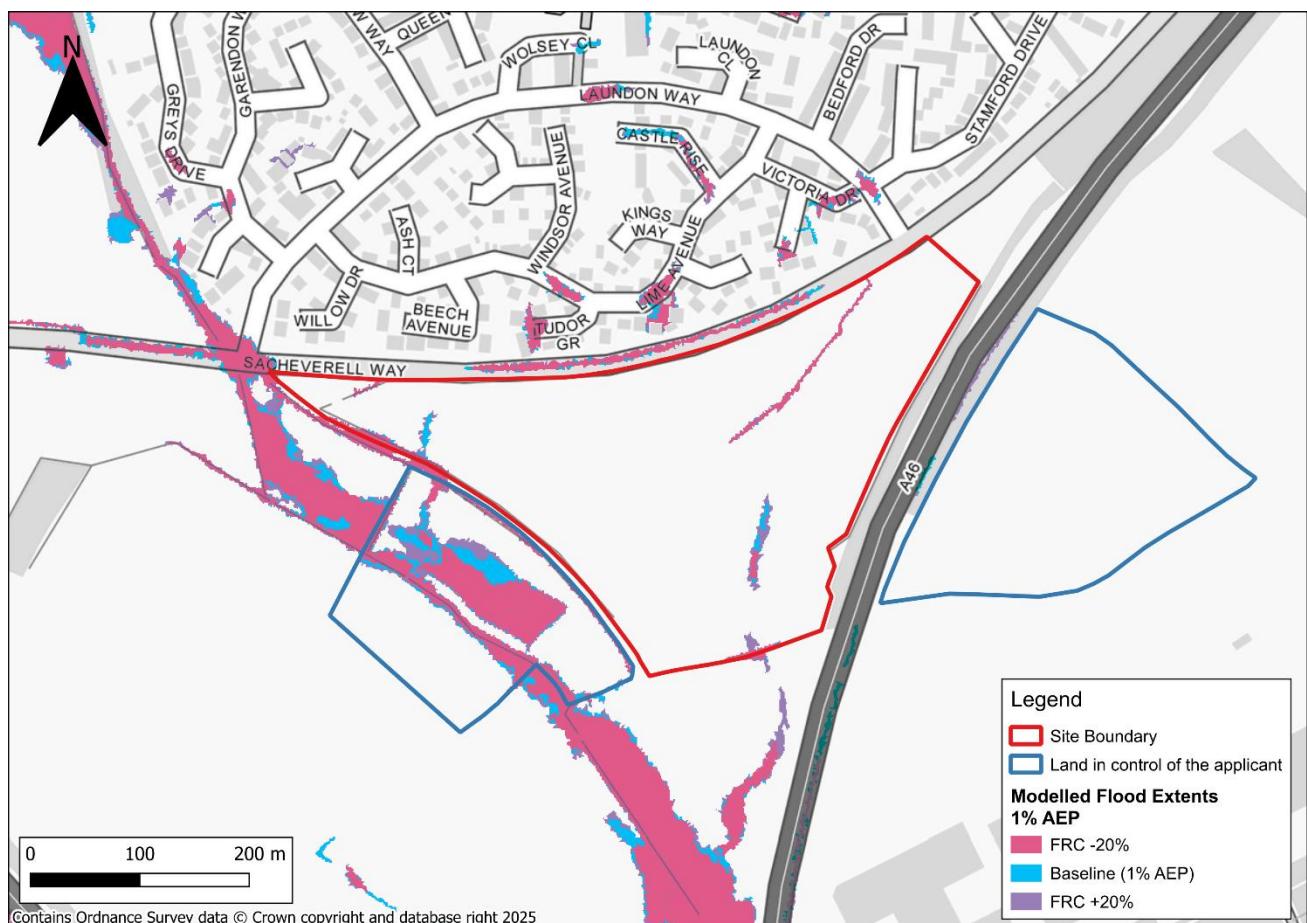


Figure D- 2: Sensitivity to changes in fixed runoff coefficient (FRC)

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