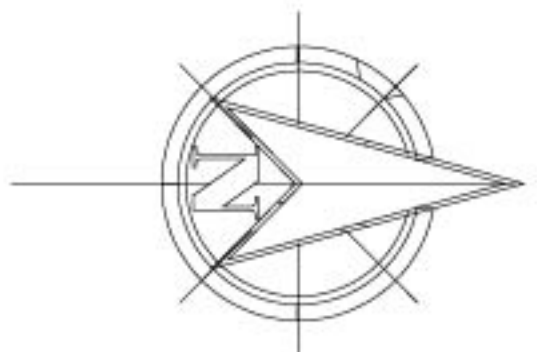


APPENDIX D

DRAINAGE CONNECTIVITY SURVEY

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There are a number of reasons why the use of a single, standard, questionnaire may be inappropriate for the study of the experience of bereavement. Firstly, the experience of bereavement is a complex phenomenon, and the use of a single questionnaire may not be able to capture the full range of experiences. Secondly, the experience of bereavement is a process, and the use of a single questionnaire may not be able to capture the changes in experience over time. Thirdly, the experience of bereavement is a social phenomenon, and the use of a single questionnaire may not be able to capture the social context of the experience. Fourthly, the experience of bereavement is a cultural phenomenon, and the use of a single questionnaire may not be able to capture the cultural context of the experience. Finally, the experience of bereavement is a personal phenomenon, and the use of a single questionnaire may not be able to capture the personal context of the experience.

DRAINAGE LEGEND

	POOL STRIPS
	RAILROADS
	RAILROADS WITH TIES
	COASTLINE
	STATE OF TENNESSEE
	STATE OF MISSISSIPPI
	MISSISSIPPI RIVER

ADDITIONAL SERVICE INFORMATION

	AR	ADDITIONAL SERVICE INFORMATION
	AR	ADDITIONAL SERVICE INFORMATION
	TR	ADDITIONAL SERVICE INFORMATION
	TR	ADDITIONAL SERVICE INFORMATION
	TR	ADDITIONAL SERVICE INFORMATION
	TR	ADDITIONAL SERVICE INFORMATION
	TR	ADDITIONAL SERVICE INFORMATION

INFORMATION SHOWS LIST OF 100 NAMES WITH CORRESPONDING UTILITIES ABBREVIATIONS

1	ALABAMA POWER	AL
2	ALBERTA ELECTRIC	AL
3	ALBERTA ELECTRIC	AL
4	ALBERTA ELECTRIC	AL
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**Geoff Perry
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The Shardbury,
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W19 5BH

1: 0922 744 900
1: 0922 744 901
e: enquiries@geoffperryassociates.co.uk

bp

SURVEYING

Client:
EICHBERGERMAN LLP

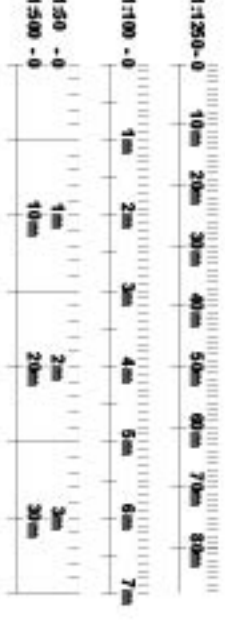
Project:
CADENT, BRICK KILN STREET
HINCKLEY, LE10 0NA

Title:
DRAINAGE SURVEY

Date: 27/01/2021 Scale: 1:500 @ A0

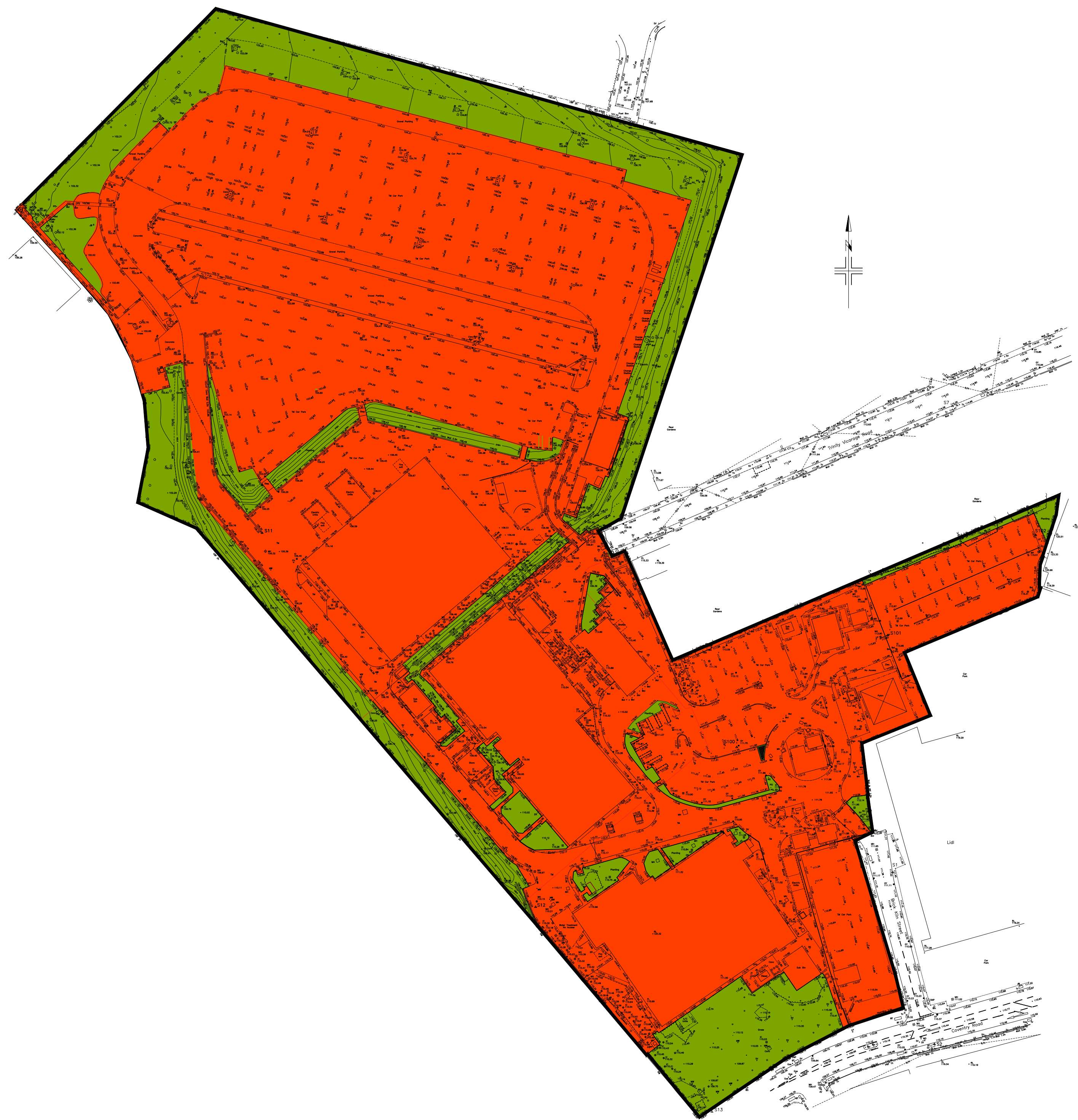
Job No. Drg No. Rev.

Scale Check:



APPENDIX E

IMPERMEABLE AREA ANALYSIS



- NOTES:
1. DO NOT SCALE THIS DRAWING. ALL DIMENSIONS MUST BE CHECKED/ VERIFIED ON SITE. IF IN DOUBT ASK.
 2. THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL RELEVANT ARCHITECTS, ENGINEERS AND SPECIALISTS DRAWINGS AND SPECIFICATIONS.
 3. ALL DIMENSIONS IN MILLIMETRES UNLESS NOTED OTHERWISE. ALL LEVELS IN METRES UNLESS NOTED OTHERWISE.
 4. ANY DISCREPANCIES NOTED ON SITE ARE TO BE REPORTED TO THE ENGINEER IMMEDIATELY.

LEGEND:

KEY	
Total Permeable Area	=12,834m ²
Total Impermeable Area	=47,713m ²
Total Site Area	=60,547m ²

P2	ADJUSTMENTS TO BOUNDARY AND TABLE	10/05/2024	DIS
P1	FIRST EXTERNAL ISSUE	07/02/2021	SR
REV	DESCRIPTION	DATE	DRAWN BY

DRAWING TITLE:

Existing Impermeable Area Plan		
DRAWING NUMBER:		REVISION:
LMX 344/100		P2
SCALE:	DRAWN:	SIZE:
1:1000	SR	A1

PROJECT:

Cadent, Brick Kiln Street
Hinckley

CLIENT:

Fisher German



Lumax

CONSULTING CIVIL &
ENVIRONMENTAL ENGINEERS

www.lumax-consulting.com
0115 8559339
info@lumax-consulting.com

APPENDIX F

CALCULATIONS

Calculated by:	Darren Loewenthal-Stewart
Site name:	Cadent
Site location:	Hinckley

This is an estimation of the greenfield runoff rates that are used to meet normal best practice criteria in line with Environment Agency guidance "Rainfall runoff management for developments", SC030219 (2013), the SuDS Manual C753 (Ciria, 2015) and the non-statutory standards for SuDS (Defra, 2015). This information on greenfield runoff rates may be the basis for setting consents for the drainage of surface water runoff from sites.

Site Details

Latitude:	52.53955° N
Longitude:	1.37995° W
Reference:	3673040100
Date:	May 12 2024 09:52

Runoff estimation approach	IH124
----------------------------	-------

Site characteristics

Total site area (ha):	6.1
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Methodology

Q _{BAR} estimation method:	Calculate from SPR and SAAR
SPR estimation method:	Calculate from SOIL type

Notes

(1) Is $Q_{\text{BAR}} < 2.0 \text{ l/s/ha}$?

When Q_{BAR} is $< 2.0 \text{ l/s/ha}$ then limiting discharge rates are set at 2.0 l/s/ha .

Soil characteristics

	Default	Edited
SOIL type:	4	4
HOST class:	N/A	N/A
SPR/SPRHOST:	0.47	0.47

(2) Are flow rates $< 5.0 \text{ l/s}$?

Where flow rates are less than 5.0 l/s consent for discharge is usually set at 5.0 l/s if blockage from vegetation and other materials is possible. Lower consent flow rates may be set where the blockage risk is addressed by using appropriate drainage elements.

Hydrological characteristics

	Default	Edited
SAAR (mm):	642	642
Hydrological region:	4	4
Growth curve factor 1 year:	0.83	0.83
Growth curve factor 30 years:	2	2
Growth curve factor 100 years:	2.57	2.57
Growth curve factor 200 years:	3.04	3.04

(3) Is $\text{SPR/SPRHOST} \leq 0.3$?

Where groundwater levels are low enough the use of soakaways to avoid discharge offsite would normally be preferred for disposal of surface water runoff.

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Greenfield runoff rates

Default

Edited

Q_{BAR} (l/s):	26.62	26.62
1 in 1 year (l/s):	22.09	22.09
1 in 30 years (l/s):	53.23	53.23
1 in 100 year (l/s):	68.4	68.4
1 in 200 years (l/s):	80.91	80.91

This report was produced using the greenfield runoff tool developed by HR Wallingford and available at www.uksuds.com. The use of this tool is subject to the UK SuDS terms and conditions and licence agreement , which can both be found at www.uksuds.com/terms-and-conditions.htm. The outputs from this tool are estimates of greenfield runoff rates. The use of these results is the responsibility of the users of this tool. No liability will be accepted by HR Wallingford, the Environment Agency, CEH, Hydrosolutions or any other organisation for the use of this data in the design or operational characteristics of any drainage scheme.

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By clicking the Accept button, you agree to us doing so.



Design Settings

Rainfall Methodology	FEH-22	Minimum Velocity (m/s)	1.00
Return Period (years)	10	Connection Type	Level Soffits
Additional Flow (%)	0	Minimum Backdrop Height (m)	0.200
CV	0.750	Preferred Cover Depth (m)	1.200
Time of Entry (mins)	5.00	Include Intermediate Ground	✓
Maximum Time of Concentration (mins)	30.00	Enforce best practice design rules	✓
Maximum Rainfall (mm/hr)	50.0		

Nodes

Name	Area (ha)	T of E (mins)	Add Inflow (l/s)	Cover Level (m)	Diameter (mm)	Easting (m)	Northing (m)	Depth (m)
Catchment A	1.920	5.00	0.0	100.000	1000	14.326	86.905	1.000
2				100.000	1500	11.758	72.747	1.288
Catchment C		5.00		100.000		34.253	52.322	1.000
3				100.000	1500	27.706	47.174	1.236
Catchment D		5.00		100.000		66.430	34.695	1.000
1				100.000	1500	56.581	25.574	1.226

Links

Name	US Node	DS Node	Length (m)	ks (mm) / n	US IL (m)	DS IL (m)	Fall (m)	Slope (1:X)	Dia (mm)	T of C (mins)	Rain (mm/hr)
1.000	Catchment A	2	14.389	0.600	99.000	98.712	0.288	50.0	525	5.08	50.0
3.000	Catchment C	3	8.329	0.600	99.000	98.764	0.236	35.3	525	5.04	50.0
2.000	Catchment D	1	13.424	0.600	99.000	98.774	0.226	59.4	525	5.08	50.0

Name	Vel (m/s)	Cap (l/s)	Flow (l/s)	US Depth (m)	DS Depth (m)	Σ Area (ha)	Σ Add Inflow (l/s)	Pro Depth (mm)	Pro Velocity (m/s)
1.000	3.173	686.8	260.2	0.475	0.763	1.920	0.0	223	2.962
3.000	3.779	818.0	0.0	0.475	0.711	0.000	0.0	0	0.000
2.000	2.910	629.9	0.0	0.475	0.701	0.000	0.0	0	0.000

Simulation Settings

Rainfall Methodology	FEH-22	Analysis Speed	Normal	Additional Storage (m³/ha)	0.0
Summer CV	0.750	Skip Steady State	x	Check Discharge Rate(s)	x
Winter CV	0.840	Drain Down Time (mins)	240	Check Discharge Volume	x

Storm Durations

15	60	180	360	600	960	2160	4320	7200	10080
30	120	240	480	720	1440	2880	5760	8640	

Return Period (years)	Climate Change (CC %)	Additional Area (A %)	Additional Flow (Q %)
2	40	0	0
30	35	0	0
100	40	0	0

Node Catchment A Time-Area Diagram

Overrides Design Area	✓	Depression Storage Area (m²)	0	Evapo-transpiration (mm/day)	0
Overrides Design Additional Inflow	x	Depression Storage Depth (mm)	0		

Applies to All storms

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-5	0.640	5-10	0.640	10-15	0.640

Node Catchment C Time-Area Diagram

Overrides Design Area	✓	Depression Storage Area (m ²)	0	Evapo-transpiration (mm/day)	0
Overrides Design Additional Inflow	x	Depression Storage Depth (mm)	0		

Applies to All storms

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-5	0.600	5-10	0.600	10-15	0.600

Node Catchment D Time-Area Diagram

Overrides Design Area	✓	Depression Storage Area (m ²)	0	Evapo-transpiration (mm/day)	0
Overrides Design Additional Inflow	x	Depression Storage Depth (mm)	0		

Applies to All storms

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-5	0.080	5-10	0.080	10-15	0.080

Node Catchment A Online Hydro-Brake® Control

Flap Valve	x	Objective	(HE) Minimise upstream storage
Replaces Downstream Link	✓	Sump Available	✓
Invert Level (m)	99.000	Product Number	CTL-SHE-0463-1456-1000-1456
Design Depth (m)	1.000	Min Outlet Diameter (m)	0.500
Design Flow (l/s)	145.6	Min Node Diameter (mm)	

Node Catchment C Online Hydro-Brake® Control

Flap Valve	x	Objective	(HE) Minimise upstream storage
Replaces Downstream Link	✓	Sump Available	✓
Invert Level (m)	99.000	Product Number	CTL-SHE-0456-1407-1000-1407
Design Depth (m)	1.000	Min Outlet Diameter (m)	0.500
Design Flow (l/s)	140.7	Min Node Diameter (mm)	

Node Catchment D Online Hydro-Brake® Control

Flap Valve	x	Objective	(HE) Minimise upstream storage
Replaces Downstream Link	✓	Sump Available	✓
Invert Level (m)	99.000	Product Number	CTL-SHE-0194-1890-1000-1890
Design Depth (m)	1.000	Min Outlet Diameter (m)	0.225
Design Flow (l/s)	18.9	Min Node Diameter (mm)	1500

Node Catchment A Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Safety Factor	2.0	Invert Level (m)	99.000
Side Inf Coefficient (m/hr)	0.00000	Porosity	1.00	Time to half empty (mins)	53



Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)
0.000	568.0	0.0	1.000	568.0	0.0

Node Catchment C Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Safety Factor	2.0	Invert Level (m)	99.000
Side Inf Coefficient (m/hr)	0.00000	Porosity	1.00	Time to half empty (mins)	51

Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)
0.000	525.0	0.0	1.000	525.0	0.0

Node Catchment D Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Safety Factor	2.0	Invert Level (m)	99.000
Side Inf Coefficient (m/hr)	0.00000	Porosity	1.00	Time to half empty (mins)	47

Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)
0.000	64.0	0.0	1.000	64.0	0.0

Results for 2 year +40% CC Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
120 minute winter	Catchment A	82	99.285	0.285	119.9	162.3249	0.0000	OK
15 minute summer	2	1	98.712	0.000	48.0	0.0000	0.0000	OK
120 minute winter	Catchment C	82	99.281	0.281	112.4	147.5002	0.0000	OK
15 minute summer	3	1	98.764	0.000	47.7	0.0000	0.0000	OK
30 minute winter	Catchment D	29	99.185	0.185	23.2	11.8310	0.0000	OK
15 minute summer	1	1	98.774	0.000	13.7	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Discharge Vol (m³)
120 minute winter	Catchment A	Hydro-Brake®	2	80.9	345.4
120 minute winter	Catchment C	Hydro-Brake®	3	77.9	325.0
30 minute winter	Catchment D	Hydro-Brake®	1	16.6	26.0

Results for 30 year +35% CC Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
60 minute winter	Catchment A	52	99.651	0.651	382.0	370.5538	0.0000	SURCHARGED
15 minute summer	2	1	98.712	0.000	142.0	0.0000	0.0000	OK
60 minute winter	Catchment C	52	99.648	0.648	358.1	340.0928	0.0000	SURCHARGED
15 minute summer	3	1	98.764	0.000	137.5	0.0000	0.0000	OK
30 minute winter	Catchment D	33	99.610	0.610	57.0	39.0241	0.0000	SURCHARGED
15 minute summer	1	1	98.774	0.000	18.9	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Discharge Vol (m³)
60 minute winter	Catchment A	Hydro-Brake®	2	145.6	624.9
60 minute winter	Catchment C	Hydro-Brake®	3	140.6	587.4
30 minute winter	Catchment D	Hydro-Brake®	1	18.9	64.2

Results for 100 year +40% CC Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
60 minute winter	Catchment A	55	99.997	0.997	528.3	566.8397	0.0000	FLOOD RISK
15 minute summer	2	1	98.712	0.000	145.6	0.0000	0.0000	OK
60 minute winter	Catchment C	55	99.996	0.996	495.2	522.9170	0.0000	FLOOD RISK
15 minute summer	3	1	98.764	0.000	140.6	0.0000	0.0000	OK
60 minute winter	Catchment D	55	99.992	0.992	66.0	63.5034	0.0000	FLOOD RISK
15 minute summer	1	1	98.774	0.000	18.9	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Discharge Vol (m³)
60 minute winter	Catchment A	Hydro-Brake®	2	145.6	869.0
60 minute winter	Catchment C	Hydro-Brake®	3	140.6	816.4
60 minute winter	Catchment D	Hydro-Brake®	1	18.9	110.8



Nodes

Name	Area (ha)	T of E (mins)	Add Inflow (l/s)	Cover Level (m)	Diameter (mm)	Easting (m)	Northing (m)	Depth (m)
Catchment A	1.920	5.00	0.0	100.000	1800	14.326	86.905	1.000
2				100.000	1500	11.758	72.747	1.288
Catchment C		5.00		100.000		34.253	52.322	1.000
3				100.000	1500	27.706	47.174	1.236
Catchment D		5.00		100.000		66.430	34.695	1.000
1				100.000	1500	56.581	25.574	1.226

Links

Name	US Node	DS Node	Length (m)	ks (mm) / n	US IL (m)	DS IL (m)	Fall (m)	Slope (1:X)	Dia (mm)	T of C (mins)	Rain (mm/hr)
1.000	Catchment A	2	14.389	0.600	99.000	98.712	0.288	50.0	525	5.08	50.0
3.000	Catchment C	3	8.329	0.600	99.000	98.764	0.236	35.3	525	5.04	50.0
2.000	Catchment D	1	13.424	0.600	99.000	98.774	0.226	59.4	525	5.08	50.0

Name	Vel (m/s)	Cap (l/s)	Flow (l/s)	US Depth (m)	DS Depth (m)	Σ Area (ha)	Σ Add Inflow (l/s)	Pro Depth (mm)	Pro Velocity (m/s)
1.000	3.173	686.8	260.2	0.475	0.763	1.920	0.0	223	2.962
3.000	3.779	818.0	0.0	0.475	0.711	0.000	0.0	0	0.000
2.000	2.910	629.9	0.0	0.475	0.701	0.000	0.0	0	0.000

Simulation Settings

Rainfall Methodology	FEH-22	Analysis Speed	Normal	Additional Storage (m³/ha)	0.0
Summer CV	0.750	Skip Steady State	x	Check Discharge Rate(s)	x
Winter CV	0.840	Drain Down Time (mins)	240	Check Discharge Volume	x

Storm Durations

15	60	180	360	600	960	2160	4320	7200	10080
30	120	240	480	720	1440	2880	5760	8640	

Return Period (years)	Climate Change (CC %)	Additional Area (A %)	Additional Flow (Q %)
2	0	0	0
30	35	0	0
100	40	0	0

Node Catchment A Time-Area Diagram

Overrides Design Area	✓	Depression Storage Area (m²)	0	Evapo-transpiration (mm/day)	0
Overrides Design Additional Inflow	x	Depression Storage Depth (mm)	0		

Applies to All storms

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-5	0.640	5-10	0.640	10-15	0.640

Node Catchment C Time-Area Diagram

Overrides Design Area	✓	Depression Storage Area (m²)	0	Evapo-transpiration (mm/day)	0
Overrides Design Additional Inflow	x	Depression Storage Depth (mm)	0		

Applies to All storms

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-5	0.600	5-10	0.600	10-15	0.600

Node Catchment D Time-Area Diagram

Overrides Design Area	✓	Depression Storage Area (m ²)	0	Evapo-transpiration (mm/day)	0
Overrides Design Additional Inflow	x	Depression Storage Depth (mm)	0		

Applies to All storms

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-5	0.080	5-10	0.080	10-15	0.080

Node Catchment A Online Depth/Flow Control

Flap Valve	x	Invert Level (m)	99.000	Design Flow (l/s)	33.1
Replaces Downstream Link	✓	Design Depth (m)	1.000		

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.001	12.890	0.176	12.890	0.687	25.780	1.000	33.100

Node Catchment C Online Depth/Flow Control

Flap Valve	x	Invert Level (m)	99.000	Design Flow (l/s)	31.1
Replaces Downstream Link	✓	Design Depth (m)	1.000		

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.001	12.110	0.179	12.110	0.702	24.210	1.000	31.100

Node Catchment D Online Hydro-Brake® Control

Flap Valve	x	Objective	(HE) Minimise upstream storage
Replaces Downstream Link	✓	Sump Available	✓
Invert Level (m)	99.000	Product Number	CTL-SHE-0106-5000-1000-5000
Design Depth (m)	1.000	Min Outlet Diameter (m)	0.150
Design Flow (l/s)	5.0	Min Node Diameter (mm)	1200

Node Catchment A Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Safety Factor	2.0	Invert Level (m)	99.000
Side Inf Coefficient (m/hr)	0.00000	Porosity	1.00	Time to half empty (mins)	

Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)
0.000	1015.0	0.0	1.000	1015.0	0.0

Node Catchment C Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Safety Factor	2.0	Invert Level (m)	99.000
Side Inf Coefficient (m/hr)	0.00000	Porosity	1.00	Time to half empty (mins)	



Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)
0.000	950.0	0.0	1.000	950.0	0.0

Node Catchment D Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Safety Factor	2.0	Invert Level (m)	99.000
Side Inf Coefficient (m/hr)	0.00000	Porosity	1.00	Time to half empty (mins)	

Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)
0.000	112.0	0.0	1.000	112.0	0.0

Results for 2 year Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
240 minute winter	Catchment A	192	99.176	0.176	58.6	178.9458	0.0000	OK
15 minute summer	2	1	98.712	0.000	12.9	0.0000	0.0000	OK
240 minute winter	Catchment C	192	99.176	0.176	55.0	167.6220	0.0000	OK
15 minute summer	3	1	98.764	0.000	12.1	0.0000	0.0000	OK
120 minute winter	Catchment D	90	99.156	0.156	10.7	17.4443	0.0000	OK
15 minute summer	1	1	98.774	0.000	3.5	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Discharge Vol (m³)
240 minute winter	Catchment A	Depth/Flow	2	12.9	339.5
240 minute winter	Catchment C	Depth/Flow	3	12.1	318.5
120 minute winter	Catchment D	Hydro-Brake®	1	4.6	30.8

Results for 30 year +35% CC Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
240 minute winter	Catchment A	232	99.688	0.688	165.6	700.3165	0.0000	SURCHARGED
15 minute summer	2	1	98.712	0.000	16.7	0.0000	0.0000	OK
240 minute winter	Catchment C	232	99.692	0.692	155.3	657.6818	0.0000	SURCHARGED
15 minute summer	3	1	98.764	0.000	15.5	0.0000	0.0000	OK
180 minute winter	Catchment D	172	99.666	0.666	25.2	74.5431	0.0000	SURCHARGED
15 minute summer	1	1	98.774	0.000	5.0	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Discharge Vol (m³)
240 minute winter	Catchment A	Depth/Flow	2	25.8	570.9
240 minute winter	Catchment C	Depth/Flow	3	24.0	532.2
180 minute winter	Catchment D	Hydro-Brake®	1	5.0	101.9

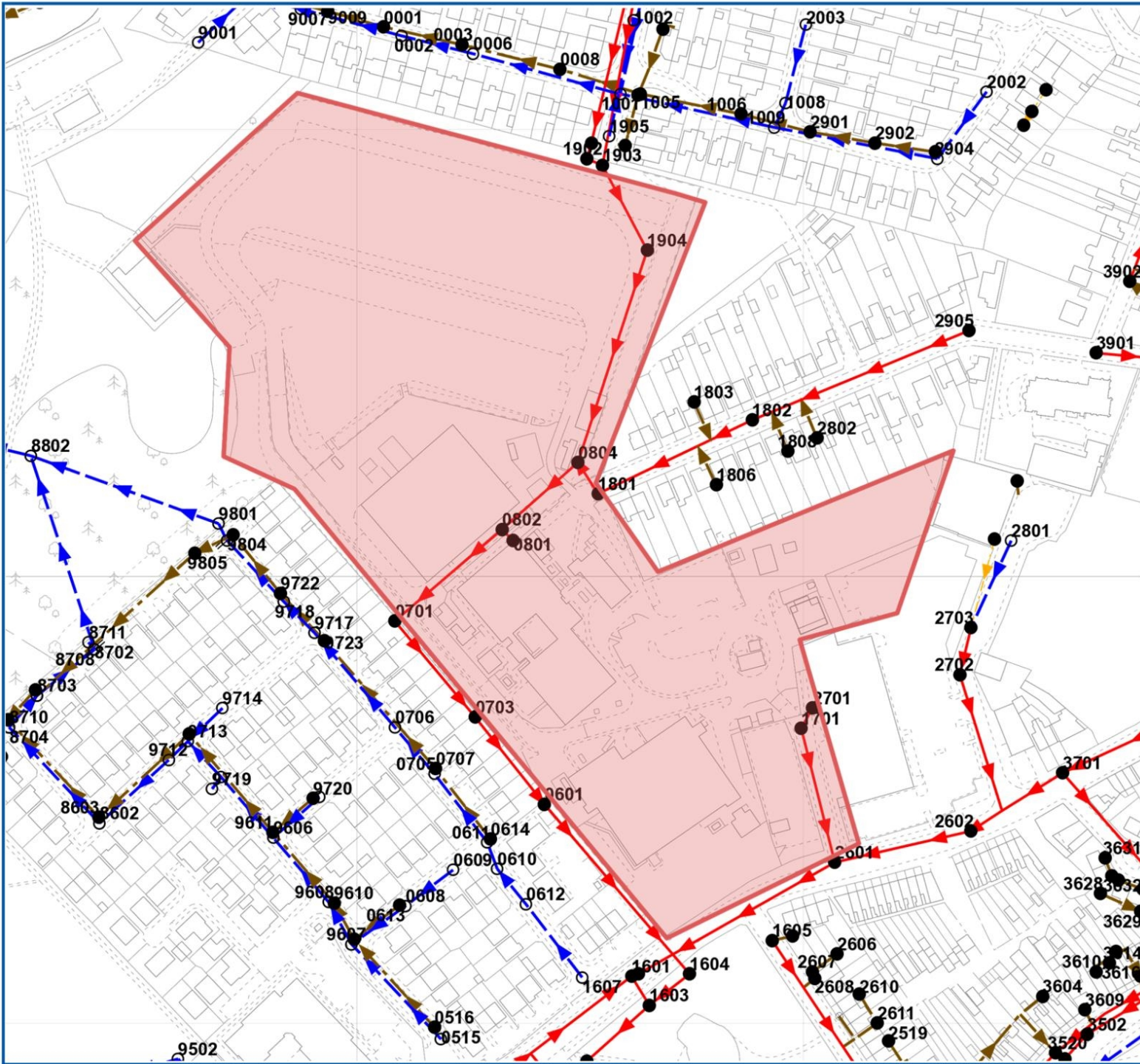
Results for 100 year +40% CC Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
240 minute winter	Catchment A	232	99.995	0.995	228.7	1012.6360	0.0000	FLOOD RISK
15 minute summer	2	1	98.712	0.000	19.8	0.0000	0.0000	OK
240 minute winter	Catchment C	232	99.999	0.999	214.4	949.2994	0.0000	FLOOD RISK
15 minute summer	3	1	98.764	0.000	18.4	0.0000	0.0000	OK
180 minute winter	Catchment D	176	99.997	0.997	34.7	111.6384	0.0000	FLOOD RISK
15 minute summer	1	1	98.774	0.000	5.0	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Discharge Vol (m³)
240 minute winter	Catchment A	Depth/Flow	2	33.0	714.1
240 minute winter	Catchment C	Depth/Flow	3	31.1	668.5
180 minute winter	Catchment D	Hydro-Brake®	1	5.0	105.9

APPENDIX G

SEVERN TRENT CONSULTATION AND SEWER MAPS



Reference	Cover Level	Invert Level Upstream	Invert Level Downstream	Purpose	Material	Pipe Shape	Max Size	Min Size	Gradient	Year Laid
SP42931801	109.0999	107.13	<UNK>	C	VC	C	225	<UNK>	0	31/12/1899 00:00:00
SP42932608	<UNK>	<UNK>	<UNK>	F	<UNK>	<UNK>	<UNK>	<UNK>	0	31/12/1899 00:00:00
SP42933610	<UNK>	0	0	F	<UNK>	<UNK>	0	0	0	31/12/1899 00:00:00
SP42931802	111.6699	110.44	107.3	C	VC	C	225	<UNK>	24.36	31/12/1899 00:00:00
SP42932902	113.73	111.67	108.76	F	VC	C	225	<UNK>	10.11	31/12/1899 00:00:00
SP42931905	107.54	106.09	<UNK>	S	VC	C	225	<UNK>	0	31/12/1899 00:00:00
SP42930802	106.6699	104.36	104.21	C	CO	C	600	<UNK>	420.87	31/12/1899 00:00:00
SP42930802	106.6699	104.57	<UNK>	C	VC	C	375	<UNK>	0	31/12/1899 00:00:00
SP42941006	109.9499	107.72	<UNK>	F	VC	C	225	<UNK>	0	31/12/1899 00:00:00
SP41939723	106.02	102.85	102.721	F	PVC	C	<UNK>	<UNK>	234.037	15/03/2019 00:00:00
SP41939801	103.306	100.07	98.13	S	CO	C	600	<UNK>	44.53	31/12/1899 00:00:00
SP42941001	107.36	105.95	<UNK>	S	CO	C	300	<UNK>	0	31/12/1899 00:00:00
SP42930705	106.319	103.829	103.667	S	URPVC	C	<UNK>	<UNK>	169.395	14/03/2019 00:00:00
SP41938603	102.904	100.594	98.797	F	URPVC	C	<UNK>	<UNK>	33.247	15/03/2019 00:00:00
SP42931602	108.16	105.84	105.83	C	VC	C	300	<UNK>	316	31/12/1899 00:00:00
SP42931603	108.54	103.29	103.28	C	CO	C	675	<UNK>	3754	31/12/1899 00:00:00
SP41938602	102.897	99.35	99.261	S	CO	C	<UNK>	<UNK>	663.191	15/03/2019 00:00:00
SP41949001	101.73	100.12	100.1	S	CO	C	225	<UNK>	1520.5	31/12/1899 00:00:00
SP42930609	105.766	104.106	103.55	S	URPVC	C	<UNK>	<UNK>	48.214	15/03/2019 00:00:00
SP42932607	<UNK>	<UNK>	<UNK>	F	<UNK>	<UNK>	<UNK>	<UNK>	0	31/12/1899 00:00:00
SP42932903	116.48	114.46	111.81	F	VC	C	150	<UNK>	10.3	31/12/1899 00:00:00
SP42933628	<UNK>	<UNK>	<UNK>	F	<UNK>	<UNK>	<UNK>	<UNK>	0	31/12/1899 00:00:00
SP41939805	103.289	101.549	100.877	F	PVC	C	<UNK>	<UNK>	86.407	15/03/2019 00:00:00
SP41939714	104.182	102.357	99.45	S	URPVC	C	<UNK>	<UNK>	7.387	15/03/2019 00:00:00
SP42933613	<UNK>	0	0	F	<UNK>	<UNK>	0	0	0	31/12/1899 00:00:00
SP42931607	107.224	105.696	104.416	S	URPVC	C	<UNK>	<UNK>	32.33	14/03/2019 00:00:00
SP42932904	116.5899	114.55	108.64	S	VC	C	225	<UNK>	12.58	31/12/1899 00:00:00
SP41939718	105.231	101.851	100.352	S	CO	C	<UNK>	<UNK>	24.949	14/03/2019 00:00:00
SP41939608	103.734	102.444	101.561	S	URPVC	C	<UNK>	<UNK>	43.05	15/03/2019 00:00:00

LEGEND

Operational Site	Foul Manhole	Transferred Foul Sewer	Private Surface Water Vacuum Sewer	Private Surface Water Lateral Drain	Sewerage Pressure Washout
Waste Water Pump	Surface Water Manhole	Disposal Pipe	Private Combined Vacuum Sewer	Private Combined Lateral Drain	Vent Column
Transferred Asset	Combined Manhole	Overflow Pipe	Private Foul Vacuum Sewer	Private Foul Lateral Drain	Waste Water Outfall
S14	Dual Manhole	Culverted Water Course	Surface Water Siphon	Transferred Surface Water Lateral Drain	Control Valve
S104	Unsurveyed Foul Manhole	Waste Internal Site Pipe	Combined Siphon	Transferred Combined Lateral Drain	Hydrocable
S102	Unsurveyed Surface Water Manhole	Sewer Service Connection	Foul Siphon	Transferred Foul Lateral Drain	Penstock
Null Private	Unsurveyed Combined Manhole	Gravity Sewer Others	Private Surface Water Siphon	Ancillary	Sewerage Isolation Valve
Null	Unsurveyed Unknown Manhole	Pressure Sewer Pipe	Private Combined Siphon	Balancing Lagoon	Sewerage Non Return Valve
None	Gravity Sewer Pipe	Surface Water Pressure Sewer	Private Foul Siphon	Grit Trap	Print200mLine
Highway Drain	Foul Gravity Sewer	Combined Pressure Sewer	S104 Surface Water Siphon	Interceptor	
Adopted Sewer	Combined Gravity Sewer	Foul Pressure Sewer	S104 Combined Siphon	Screen	
Storage	Surface Water Gravity Sewer	S104 Surface Water Pressure Sewer	S104 Foul Siphon	Chamber	
DS	S104 Surface Water Gravity Sewer	S104 Combined Pressure Sewer	Combined Unsurveyed Pipe	Flushing Chamber	
Off-Line Waste Water Storage	S104 Combined Gravity Sewer	S104 Foul Pressure Sewer	Foul Unsurveyed Pipe	Scallaway	
On-Line Waste Water Storage	S104 Foul Gravity Sewer	Private Surface Water Pressure Sewer	Private Combined Pressure Sewer	Overflow	
Wet Well	Private Surface Water Gravity Sewer	Private Foul Pressure Sewer	Surface Water Unsurveyed Pipe	Fitting	
Waste Water Process Structure	Private Combined Gravity Sewer	Surface Water Unsurveyed Pipe	Combined Unsurveyed Pipe	Blind Shaft	
Sewage Treatment Point	Surface Water Unsurveyed Pipe	Foul Vacuum Sewer	Combined Lateral Drain	Facility Connector	
Sewage Treatment Structure	Combined Unsurveyed Pipe	Combined Vacuum Sewer	Foul Lateral Drain	Head Node	
Sludge Treatment Point	Foul Unsurveyed Pipe	S104 Surface Water Vacuum Sewer	S104 Surface Water Lateral Drain	Lamphole	
Sludge Treatment Structure	S104 Combined Vacuum Sewer	S104 Combined Lateral Drain	S104 Foul Vacuum Sewer	Sewerage Air Valve	
Manhole	S104 Foul Lateral Drain	Sewerage Hatch Box		Sewerage Chemical Injection Point	
Twin Manhole					

MATERIALS

-	- NONE
AC	- ASBESTOS CEME
BR	-

From: [Network Solutions](#)
To: [Kate Loewenthal](#)
Subject: RE: Brick Kiln Lane Your Ref 8433384
Date: 08 May 2024 12:47:21
Attachments: [image003.png](#)
[image004.png](#)
[Sewer Record Plan.pdf](#)

ST Classification: OFFICIAL PERSONAL

Hi Kate, I am good thank you hope you are too.

Gravity foul flows at 2DWF for 195 properties gives an approximate rate of 3l/s. I can confirm that a connection to MH0802 on the 600mm combined network can accommodate these flows (subject to 106 approval).

In relation to the surface water network, I can confirm that the sewers you wanted to connect to previously have now been fully adopted. A connection should therefore be pursued to the 900mm surface water network in Reith Close as originally intended. Please see updated sewer records attached.

Kind Regards

Emma Nowak

Senior Evaluation Technician

Network Solutions

Developer Services

Mobile : 07970361864

Emma.Nowak@severntrent.co.uk

-

For further information on guidance and applications please follow the link below:

<https://www.stwater.co.uk/building-and-developing/regulations-and-forms/application-forms-and-guidance/>

We have listened to our customers & local communities and as a result, Severn Trent have made a pledge to transform and protect the health of our rivers. For more information please follow the link below:

<https://www.stwater.co.uk/get-river-positive/our-river-pledges/>

-

WONDERFUL ON TAP



From: Kate Loewenthal <Kate.Loewenthal@lumax-consulting.com>

Sent: Wednesday, May 8, 2024 10:47 AM

To: Emma Nowak <Emma.Nowak@severntrent.co.uk>

Subject: RE: Brick Kiln Lane Your Ref 8433384

You don't often get email from kate.loewenthal@lumax-consulting.com. [Learn why this is important](#)

Caution: This is an external email originating outside Severn Trent.
Think before you click on links or open attachments.

Hi Emma

Hope you are well.

In relation to this project, the proposal has now been altered to develop the site as purely residential with up to 195 properties and we are adapting our FRA and Drainage report to assist with a new Outline Planning Application.

Could you please confirm that STW would still have capacity for the increase from 150 properties to a maximum of 195 properties?

Furthermore, in the email below, you mentioned MH9806 is still currently under a 104 adoption agreement and would therefore need approval from the current owners. As this was now three years ago, can you please confirm if this MH has now been adopted by STW, or if still under the adoption agreement?

Thanks

All the best

Kate

Kate Loewenthal
Lumax Consulting

Mob : 07736 341306 | Tel : 0115 855 9339 | Fax : 0115 855 9349



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From: Nowak, Emma <Emma.Nowak@severntrent.co.uk>

Sent: Thursday, February 25, 2021 5:21 PM

To: Kate Loewenthal <Kate.Loewenthal@lumax-consulting.com>

Subject: RE: Brick Kiln Lane Your Ref 8433384

ST Classification: OFFICIAL PERSONAL

Hi Kate,

Thanks for your email. Please see the attached sewer record plan which shows the existing SW public sewer with MH9801. Could you please confirm the location of M8800 as your email below states that this is on the SW run but then further down your email you refer to it on the combined network. Could you please clarify? MH9806 is still currently under a 104 adoption agreement and would therefore need approval from the current owners. A SW connection at public MH9801 would be acceptable subject to 106 approval.

Please keep up to date with regards the foul drainage, a pumped solution may require sewer modelling to ascertain the impact of pumped flows on the receiving network. Please submit pumped proposals as soon as possible so this can be assessed.

Kind Regards,

Emma Nowak

Senior Evaluation Technician

Asset Protection East

Wholesale Operations

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<https://www.stwater.co.uk/building-and-developing/regulations-and-forms/application-forms-and-guidance/>

WONDERFUL ON TAP



From: Kate Loewenthal <Kate.Loewenthal@lumax-consulting.com>

Sent: 25 February 2021 14:35

To: Nowak, Emma <Emma.Nowak@severntrent.co.uk>

Subject: RE: Brick Kiln Lane Your Ref 8433384

Hi Emma,

Thanks for your response. Unfortunately I cannot locate MH 9801 on the sewer map that we received (see attached) However, during an initial drainage investigation survey of the site, a private drain was identified discharging to the nw of the site. It is likely that this will be upgraded and connected by gravity to the surface water run between MH 9806 and MH 8800. Once planning has been approved and prior to construction commencing, we have recommended a more detailed cctv survey of this drain and potential connection points and a S106 will be completed at that point.

Our recommendations regarding foul drainage are either a pumped solution to discharge into the main combined sewer crossing the site at MH 0802 or alternatively apply for a requisition enabling a connection from the north west of the site, to the combined sewer at MH 8800 in the centre of the parkland. At this point in the planning process, the design for either the layout or the drainage is not finalised but we will keep you updated.

Please let me know if you have any further comments. Thanks.

Kind regards

Kate

Kate Loewenthal
Lumax Consulting

Mob : 07736 341306 | Tel : 0115 855 9339 | Fax : 0115 855 9349



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From: Nowak, Emma <Emma.Nowak@severntrent.co.uk>
Sent: 24 February 2021 16:06
To: Kate Loewenthal <Kate.Loewenthal@lumax-consulting.com>
Subject: RE: Brick Kiln Lane Your Ref 8433384

ST Classification: OFFICIAL PERSONAL

Hi Kate,

I can confirm that 150 houses equates to approximately 2.3 l/s for **foul flows only**. As stated in the initial developer enquiry, a foul only connection at the existing MH0802 on the 600mm CWS would be acceptable to STW subject to 106 approval. Could you please confirm that SW flows will be going to the SWS sewer north west of the site at MH9801 as stated in the initial response

(please see attached for ease of reference).

I look forward to your response.

Kind Regards,

Emma Nowak

Senior Evaluation Technician

Asset Protection East

Wholesale Operations

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WONDERFUL ON TAP



From: Kate Loewenthal <Kate.Loewenthal@lumax-consulting.com>

Sent: 19 February 2021 14:21

To: Nowak, Emma <Emma.Nowak@severntrent.co.uk>

Subject: RE: Brick Kiln Lane Your Ref 8433384

Hi Emma,

Hope you are well.

The developer for this site has increased the number of new builds to 150. These will all still be in the carpark area to the north of the site, north of the main 600mm sewer crossing the middle of the site. Could you please confirm that STW has capacity in the network for this increased number of dwellings.

Thanking you in anticipation,

Kind regards

Kate

Kate Loewenthal
Lumax Consulting

Mob : 07736 341306 | Tel : 0115 855 9339 | Fax : 0115 855 9349



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From: Nowak, Emma <Emma.Nowak@severntrent.co.uk>
Sent: 23 November 2020 19:53
To: Kate Loewenthal <Kate.Loewenthal@lumax-consulting.com>
Subject: RE: Brick Kiln Lane Your Ref 8433384

ST Classification: OFFICIAL PERSONAL

Hi Kate,

Thank you for your email below regarding the proposed development at Brick Kiln Lane. With regards to the sewers crossing the site and the relevant easements, I can confirm that the sewer crossing the middle of the site is 600mm in size, and the sewer running down Brick Kiln Street within the south western boundary of your site is 225mm. Secondly, relating to your question concerning the capacity of the sewers, I can confirm that the proposed flow rate of 0.9 l/s was in relation to the proposed **60** dwellings and calculated again to double check. Therefore I can confirm there is capacity in the network to take the proposed 60 houses. The statement regarding the '12 dwellings' was a typo made in error and in no relation to the flow rate made within the statement. Please find the amended letter attached which reflects the proposed 60 dwellings drainage response.

I hope this helps.

Kind Regards

Emma Nowak

Senior Evaluation Technician
Asset Protection East
Wholesale Operations

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<https://www.stwater.co.uk/building-and-developing/regulations-and-forms/application-forms-and-guidance/>

WONDERFUL ON TAP



From: Kate Loewenthal <Kate.Loewenthal@lumax-consulting.com>

Sent: 19 November 2020 11:40

To: Net Dev East <Net.Dev.East@severntrent.co.uk>

Subject: Brick Kiln Lane Your Ref 8433384

To whom it may concern

Thank you for the attached information on a site at Brick Kiln Lane Hinckley in response to our DSE. Could you please clarify a couple of points.

- Two combined sewers cross the site. In the Developer Enquiry Response they are mentioned in regard to the required easements, and they are two different sized sewers requiring 2 different easements. However, it is not indicated on the sewer map or in the written response, which sewer is which size. I am assuming from the comment in the Foul Drainage section, that the sewer crossing the centre of the site is the larger of the two. Could you please clarify this.
- In the Developer Enquiry Response, the Foul Drainage section says '*Sewer records show the closest point of connection for foul flows is at the existing 600mm public combined sewer crossing the site. A foul connection for the proposed **12** houses would be approximately 0.9 l/s.*' However, in the DSE and in the heading of the Developer Enquiry Response, it is stated that the development is for approximately 60 residential dwellings. Could you please confirm that there is still capacity for this number.

It would be really appreciated if you could come back to me quickly on this, as our deadline for submitting the report is approaching.

Thanking you in anticipation

Kate Loewenthal
Lumax Consulting

Mob : 07736 341306 | Tel : 0115 855 9339 | Fax : 0115 855 9349



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